

Appendix A3.1 Assessment of Domestic and Non-Domestic Load of Proposed Regional Wastewater Treatment Plant

JACOBS[®]



Greater Dublin Drainage

Irish Water

Assessment of Domestic & Non-Domestic Load on Proposed Regional WwTP

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1. Introduction

1.1 Title

The official name of the project is *Greater Dublin Drainage – Regional Wastewater Treatment Plant, Marine Outfall & Orbital Drainage System*.

1.2 Client

At the commencement of the GDD project in 2011, the 34 local authorities of Ireland were responsible for providing water and wastewater services and infrastructure in their respective administrative areas under the Local Government Act 2001. Jacobs Engineering Ireland Ltd. (Jacobs) in association with TOBIN Consulting Engineers (Tobin) were appointed by Fingal County Council (FCC) (as the Contracting Authority on behalf of Meath, Kildare, Dun Laoghaire / Rathdown and South Dublin County Councils and Dublin City Council) in March 2011 to act as Project Engineering Consultant on this project.

However, under the Water Services (No. 2) Act 2013, the responsibilities of FCC on this project were transferred to Irish Water/ Uisce Éireann (IW), a subsidiary of Ervia (formerly Bord Gáis Éireann) on 1st January 2014. At that point the existing Jacobs / Tobin contract was novated across to IW who thereafter became the Project Client.

1.3 Previous Reference Studies

- *Greater Dublin Strategic Drainage Study (GSDSDS) completed in April 2005, and*
- *Strategic Environmental Assessment of the Greater Dublin Strategic Drainage Study (SEA of GSDSDS) completed in May 2008.*

1.4 Project Stages

The Project is divided into a number of stages as follows:

- Sub – Stage (a): Project Inception
- Sub - Stage (b): Alternative WwTP Site Assessment(ASA)/Pipeline and Marine Route Selection Report
- Sub – Stage (c): Concept Design Report
- Sub – Stage (d): Environmental Impact Assessment
- Sub – Stage (e): Wayleave / Land Acquisition
- Sub – Stage (f): Additional Reports
- Sub – Stage (g): Planning Process
- Sub – Stage (h): Any Other Work

1.5 Objectives

The primary project objective is to provide a long-term drainage solution that shall cater for existing & future development in the Greater Dublin Area (GDA) by implementing the recommendations of the Greater Dublin Strategic Drainage Study (GDSDS) Final Strategy and the Strategic Environmental Assessment (SEA) of the GDSDS. This requires an integrated programme of works to provide sustainable wastewater treatment and sewer network capacity in the region.

The key objectives of the GDD are to safely deliver through the planning process a:

- *Regional Wastewater Treatment Plant (WwTP) and associated marine outfall located at a site in the Northern part of the Greater Dublin Area (GDA), and*
- *an orbital sewer, associated pumping stations and outfall pipeline linking the regional WwTP to the existing regional sewer network and to provide for future connections for identified developing areas within the catchment.*

In April 2013, a review of Fingal's Sludge Management Plan (SMP) was completed which recommended that Fingal develop a single Sludge Hub Centre (SHC) to treat all wastewater sludges arising in Fingal and that this SHC should be co-located with the proposed Regional WwTP.

Irish Water reviewed this proposal as part of its national wastewater sludge management plan¹ and considered it to provide the most appropriate option for a sludge hub in Fingal.

The SHC is an element of the treatment process provided by the proposed WwTP, therefore any further reference to the WwTP includes the SHC.

1.6 Commencement Date

The official commencement date of the project is set as the 14th March 2011.

¹ National Sludge Management Plan, Irish Water; 2016

2. Background and Purpose of Report

2.1 Introduction

The Greater Dublin Drainage (GDD) project has its origins in the Greater Dublin Strategic Drainage Study (GSDSDS)², which was a major region wide strategic study conducted between 2001 and 2005 to examine and report on the medium and long-term urban drainage needs. The GSDSDS was commissioned as a result of the broadening gap between the developing load in the Greater Dublin Area (GDA) and the maximum load which can be delivered to and treated at the existing Wastewater Treatment Plants (WwTPs) in the catchment and primarily at Ringsend WwTP. In order to address this, the *GSDSDS Final Strategy Report, April 2005*, made detailed recommendations on wastewater infrastructure requirements, which included the optimisation of the capacity of existing WwTPs and networks for near-term requirements, coupled with the development of new infrastructure to meet growth in the medium and long-term.

The key findings of the GSDSDS were the subject of a Strategic Environmental Assessment (SEA), which was completed in 2008. The SEA endorsed the fundamental concept and scale of the GSDSDS Final Strategy Report, but cautioned that the site selection needed to take place in a process of rigorous appraisal of alternatives. The key recommendations of the SEA are as follows:

- The upgrade of all wastewater treatment plants (WwTPs) in the region, including Ringsend, to their ultimate capacity;
- Construction of a new regional WwTP, associated orbital sewer and marine outfall in the northern part of the Greater Dublin Area (GDA);
- Completion of a rigorous 4-stage 'Alternative Sites Assessment Study' to determine the preferred location for the Regional WwTP;
- The associated orbital sewer and marine outfall route to be defined; and
- A suite of mitigation measures and a Monitoring Programme to be undertaken during the construction and operation of the project.

In the absence of the implementation of the above proposed drainage strategy the SEA considered that inadequate wastewater treatment and drainage management would result in development constraints within the area covered by the strategy. Thus, Local Authorities (LAS) would be inhibited from effectively implementing their respective County and City Development Plans.

2.1.1 GSDSDS – Assessment of Future Growth

In the preparation of the GSDSDS Final Strategy Report a Population and Land Use Study was carried out to determine land usage and planning requirements within the study area so that the capability of existing drainage infrastructure could be assessed against projected future flows and loads. The final report of this Population and Land Use study was published in March 2003³. This report estimated the future wastewater treatment requirements, defined on a WwTP catchment basis rather than

² The GSDSDS was proposed by the Dublin Region Local Authorities in the Greater Dublin Area and was supported by the Department of the Environment, Heritage and Local Government. Dublin City Council was appointed as the contracting authority for the study which was conducted by the Dublin Drainage Consultancy.

³ GSDSDS – Population and Land Use – Final Report, March 2003 (Ref.GSDSDS/NE02057/094v2)

administrative boundaries within the GDA for three design scenarios: 2002, 2011 and 2031, with a view to using these projections as the basis for determining the extent of additional wastewater infrastructure required to meet this future demand.

The first scenario (2002) represented the then baseline or existing situation. The second (2011) corresponded to the planning horizon of the Strategic Planning Guidelines at that time. The third (2031) represented a long-term horizon appropriate for the planning of major strategic infrastructure.

The projected wastewater loads from the GSDS Final Strategy Report are set out in Table 2.1 below.

Table 2.1: Organic Loading on existing WwTPs (Source Table 10.4 of the GSDS Final Strategy Report)

WwTP	Current (2002) Design PE	Ultimate Design PE	PE Load Existing (2002)	PE Load 2011	PE Load 2031	Comments
Ringsend	1,640,000	1,905,000 to 2,160,000	1,750,000 to 1,900,000	2,402,603	2,813,901	Extend to 2.16M PE a.s.a.p.
Shanganagh Bray	167,400	200,000 to 240,000	106,900	162,505	249,016	Phase 1 to 180,000 PE. Extend after 2011
Osberstown	80,000	130,000	57,533	98,152	154,088	Extend towards 2011
Leixlip	90,000	130,800	68,189	100,343	183,378	Extend after 2011
Portrane	35,000	65,000	14,531	30,249	45,650	Extend towards 2011
Malahide	20,000	25,000	16,089	16,669	23,236	Extend after 2011
Balbriggan & Skerries	30,000	70,000 to 90,000	19,008	55,852	90,863	Extend towards 2011
Swords	60,000	90,000	34,254	75,241	109,567	Extend towards 2011
Totals	2,122,400	2,615,800 to 2,930,800	2,066,534 to 2,216,534	2,941,614	3,669,698	

As evident in Table 3.1 the GSDS determined that the 2002 wastewater load, in terms of combined residential population, commercial, institutional and industrial sources, exceeded the installed wastewater treatment capacity in the GDA at that time. Furthermore, it determined that even with the expansion of each of the existing WwTPs to their ultimate design capacity the projected combined growth (residential population, commercial, institutional and industrial sources) in the GDA would exceed the treatment capacity provided by these WwTPs.

The GSDS also determined that the ability to expand the treatment capacity at each of the WwTPs beyond their ultimate design capacity was limited by either site and/or receiving water constraints at

each WwTP. It also found that there was limited capacity in the existing drainage networks to accept flows from future development, noting significant overloading of sewers, deficiencies at combined sewer overflows and increased risk of sewer flooding throughout the network. Constraints on further upgrade works to address these capacity issues, particularly in the network serving Ringsend, included the intensity of urban development, associated utilities and traffic. Considering the scale of the network upgrade work required GDSDS described them as representing:

“a major engineering challenge, particularly where large diameter pipelines have to be constructed in roadways already saturated with utility services and traffic. Even with tunnel construction, the accommodation of shafts and protection of existing works, traffic management and general management of environmental impacts would be extremely difficult.”

The GDSDS therefore determined that the optimum solution to the above treatment and network capacity deficits was to provide new wastewater treatment capacity in north County Dublin.

The proposed new wastewater treatment plant would primarily augment the treatment capacity provided at Ringsend WwTP through the diversion of flow and load out of the Ringsend catchment to the new WwTP thereby freeing up capacity at Ringsend WwTP and in its contributing network. Furthermore, the proposed new WwTP would also augment other WwTPs in the GDA through diversion of flow & load in excess of their ultimate treatment capacity to the proposed new WwTP.

2.2 Purpose of this Report

The strategies proposed by GDSDS to meet the GDA drainage infrastructural requirements, at the 2011 and 2031 design horizons adopted in that Study, were predicated on population projections base-lined on the 2002 Census, with non-domestic and trade effluent data built up from considerations of sub-catchment planning potential. The detailed Population and Land Use Study, undertaken as part of the GDSDS and reported on in March 2003⁴ did not foresee the large inward migration that occurred, post 2004, following expansion of the EU nor did it foresee the extent of emigration that occurred during the economic recession between 2008 - 2013. It is clear that the economic landscape has altered markedly since the GDSDS Population and Land Use Study was undertaken. It is therefore prudent to re-examine population and load projections within the GDA to assess whether the recommendations of GDSDS and its SEA remain valid.

The release of the results from Census 2016 presents an opportunity on which to base this review of population and load projections and to re-baseline the proposed growth projections to 2016 data. The 2013 updates by the Central Statistics Office (CSO) of the Population and Labour Force Projections, 2016 – 2046 (published April 2013) and the Regional Population Projections 2016 - 2031 (published December 2013) also permitted a re-examination of population growth rates in the GDA, with particular emphasis on the contributing catchment to Ringsend WwTP.

In addition, a ‘Demographic Study’⁵ was commissioned by Irish Water in May 2014 as part of the Water Supply Project Eastern and Midlands Region (WSP). The objective of this study was to examine a range of population projections out to 2050, to be used as the basis for the estimation of

⁴ GDSDS – Population and Land Use – Final Report, March 2003 (Ref.GDSDS/NE02057/094v2)

⁵ Water Supply Project Dublin Region: Summary of Demographic Projections, AOS Planning, June 2014

water demand for the WSP. The study sets out regional population projections for the Planning Regions and the State to 2050. The projections were prepared as per the last census for the base year 2011, with projections for the years 2021, 2026, 2031, 2041, 2046 and 2050. Years 2031 and 2046 respectively, represent the furthest dates used for the CSO Regional and State Population Projections.

The purpose of this report (*GDD Assessment of Domestic & Non-Domestic Load Report*) is to:

- i. Re-examine population and required treatment capacity projections within the GDA to assess whether the recommendations of GSDSDS and its SEA remain valid.
- ii. Determine the requirement, both in terms of timing and treatment capacity, for the proposed Regional WwTP.

This determination is closely linked to the capacity of the existing plant at Ringsend and to the limited capacity and significant constraints in the existing drainage network serving Ringsend WwTP.

Given the passage of time since the GSDSDS Final Strategy Report was published the baseline year for the review of future growth has been set at 2016. A design year horizon of 2050 has also been adopted as this represents a long-term horizon appropriate for the planning of major strategic infrastructure based on the anticipated commissioning date of the proposed Project.

2.3 Study Area

The study area for the GDD project is illustrated in Figure 2.1 in Appendix B.

2.4 Contributing Catchment to Ringsend WwTP

The contributing catchment to Ringsend WwTP is illustrated in Figure 2.2 in Appendix B.

2.5 Potential Contributing Catchment to Regional WwTP

The GDD project will primarily serve the northern, north-western and western sub-catchments of the current Ringsend catchment. It is proposed to divert flow and load from these sub-catchments to the proposed Regional WwTP. The timing of these diversions will depend on the development of load in the Ringsend catchment and in each of the individual sub-catchments over time.

The sub-catchments located on the northern, north-western and western fringe of the Ringsend catchment are indicated in Figures 2.3 – 2.5 in Appendix B and comprise of;

- *The Blanchardstown (9C Sewer) sub-catchment of Ringsend WwTP (includes the Meath towns & villages of Ashbourne, Ratoath, Kilbride, Dunboyne & Clonee),*
- *The North Dublin (North Fringe Sewer & North Dublin Drainage Scheme (NDDS) Sewer) sub-catchment of Ringsend WwTP,*
- *The South Dublin – Lucan/Clondalkin (9B Sewer) sub-catchment of Ringsend WwTP.*

Additional catchments in the GDA, which may also influence future required treatment capacity of the new Regional WwTP, through diversion of flows & load in excess of ultimate treatment capability of

the individual wastewater treatment plants in these catchments, are indicated in Figure 2.6 in Appendix B and comprise;

- *Swords WwTP Catchment,*
- *Malahide WwTP Catchment,*
- *Lower Liffey Valley (Leixlip WwTP) Catchment (Includes Leixlip, Celbridge, Maynooth, Kilcock and Straffan),*
- *Upper Liffey Valley (Osberstown WwTP) Catchment (Includes Naas, Prosperous, Clane, Sallins, Kill, Johnstown, Newbridge, Athgarvan and Kilcullen).*

3. Existing Population and Future Growth Rates

3.1 Current Population within the GDA

The Census of 2016 indicates that the population in the Greater Dublin Area (GDA) in April 2016 was 1,907,332 persons. This is an increase of 5.72% from the 2011 population of 1,804,156 persons, which is ahead of the national increase of 3.8%, and represents an annual average percentage growth rate of 1.118% across the GDA.

The Census results for the GDA and its constituent regions and local authorities are shown in Table 3.1. Figures from the 2011 Census are also provided.

Table 3.1 Population Numbers in GDA from Census 2011 & 2016

Local Authority Area	2011	2016	Population Increase	Percentage Increase	Annual Average Growth Rate
	Final Census 2011 Results	Final Census Results	2011 - 2016	2011 - 2016	%
Dublin City	527,612	554,554	26,942	5.106	1.001
Dun Laoghaire - Rathdown	206,261	218,018	11,757	5.700	1.115
Fingal	273,991	296,020	22,029	8.040	1.559
South Dublin	265,205	278,767	13,562	5.114	1.002
Sub - Total for Dublin Region	1,273,069	1,347,359	74,290	5.836	1.141
Kildare	210,312	222,504	12,192	5.797	1.133
Meath	184,135	195,044	10,909	5.924	1.158
Wicklow	136,640	142,425	5,785	4.234	0.833
Sub - Total for Mid - East Region	531,087	559,973	28,886	5.439	1.065
GDA Total	1,804,156	1,907,332	103,176	5.719	1.118

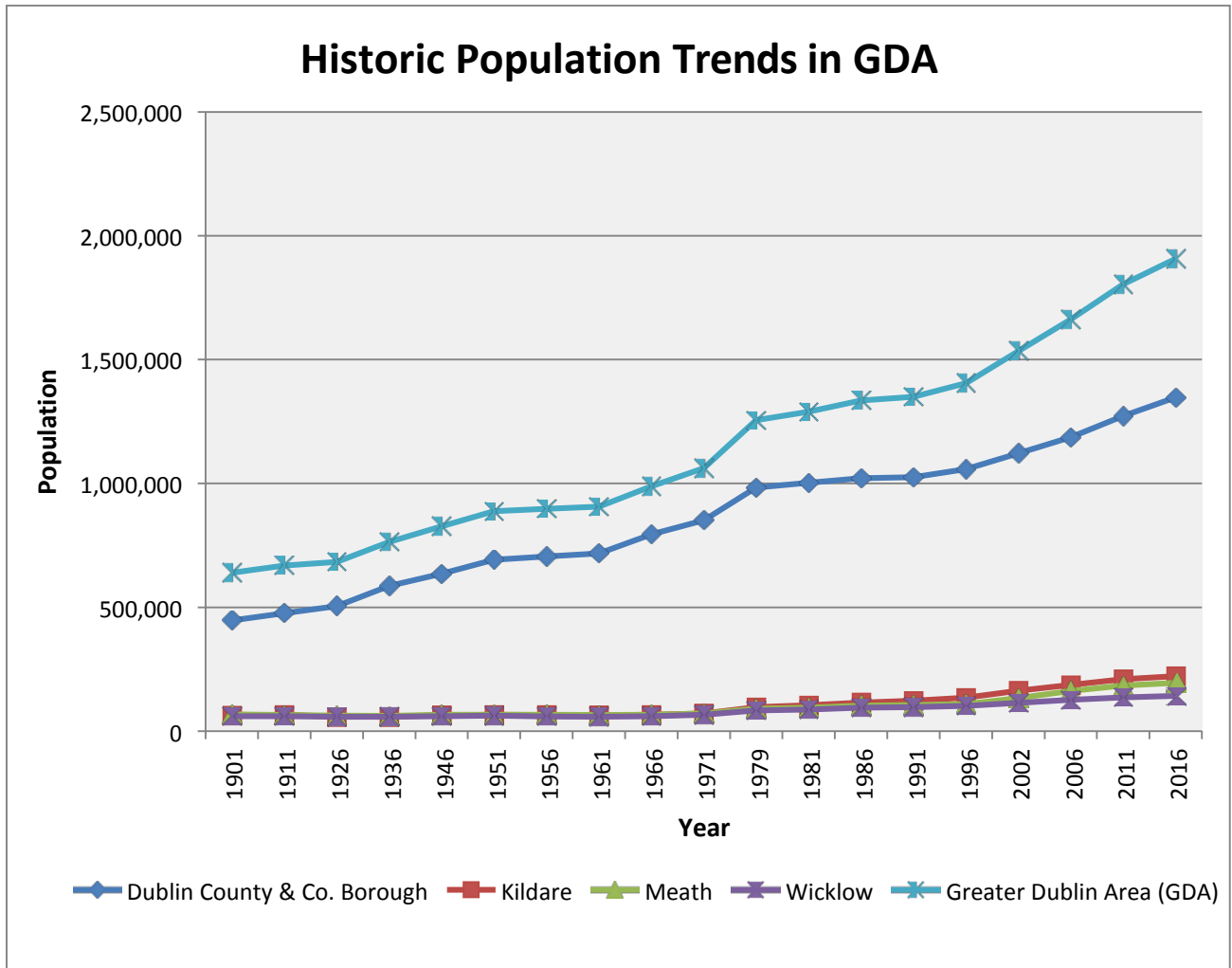
3.2 Historic Population Trends within the GDA

The population in the GDA at each Census year between 1901 and 2016 is illustrated in Table 3.2 and graphed in Figure 3.1.

Table 3.2 Historic Population Trends in GDA; 1901 - 2016

Census Year	Dublin County & Co. Borough	Kildare	Meath	Wicklow	Greater Dublin Area (GDA)
1901	448,206	63,566	67,497	60,824	640,093
1911	477,196	66,627	65,091	60,711	669,625
1926	505,654	58,028	62,969	57,591	684,242
1936	586,925	57,892	61,405	58,569	764,791
1946	636,193	64,849	66,232	60,451	827,725
1951	693,022	66,437	66,337	62,590	888,386
1956	705,781	65,915	66,762	59,906	898,364
1961	718,332	64,420	65,122	58,473	906,347
1966	795,047	66,404	67,323	60,428	989,202
1971	852,219	71,977	71,729	66,295	1,062,220
1979	983,683	97,185	90,715	83,950	1,255,533
1981	1,003,164	104,122	95,419	87,449	1,290,154
1986	1,021,449	116,247	103,881	94,542	1,336,119
1991	1,025,304	122,656	105,370	97,265	1,350,595
1996	1,058,264	134,992	109,732	102,683	1,405,671
2002	1,122,821	163,944	134,005	114,676	1,535,446
2006	1,187,176	186,335	162,831	126,194	1,662,536
2011	1,273,069	210,312	184,135	136,640	1,804,156
2016	1,347,359	222,504	195,044	142,425	1,907,332

Figure 3.1 Historic Population Trends in GDA



3.2.1 Historic Average Annual Growth Rates

Average annual percentage growth rates for Dublin County & County Borough, Counties Kildare, Meath & Wicklow and the GDA are shown in Table 3.3.

Table 3.3 Average Annual % Growth Rates

Period	Dublin County & Co. Borough	Kildare	Meath	Wicklow	Greater Dublin Area (GDA)
1901 - 2016	0.962	1.095	0.927	0.743	0.954
1911 - 2016	0.993	1.155	1.051	0.815	1.002
1926 - 2011	1.095	1.505	1.264	1.011	1.146
1936 - 2016	1.044	1.697	1.455	1.117	1.149
1946 - 2011	1.078	1.777	1.555	1.232	1.200
1951 - 2016	1.028	1.877	1.673	1.273	1.182

Period	Dublin County & Co. Borough	Kildare	Meath	Wicklow	Greater Dublin Area (GDA)
1961 - 2011	1.150	2.279	2.015	1.632	1.362
1971 - 2016	1.023	2.540	2.248	1.714	1.309
1981 - 2016	0.846	2.193	2.064	1.403	1.123
1991 - 2016	1.099	2.411	2.494	1.537	1.390
2002 - 2016	1.311	2.206	2.717	1.560	1.561
2011 - 2016	1.141	1.133	1.158	0.833	1.118

3.3 Future Projections of Population within the GDA

3.3.1 Introduction

In December 2013, the Central Statistics Office (CSO) published the 'Regional Population Projections' 2016-2031. This publication provides a regional breakdown of the CSO national projections and contains population projections for the eight Regional Authority areas in Ireland for 2016-2031. The assumptions used in the national model (fertility, mortality and international migration) have been regionalised based on recent historical data, and regional migration, but remain within the overall constraint of the national population projections. The regional data is presented in combinations of fertility and migration assumptions.

In May 2014, Irish Water commissioned a 'Demographic Study'⁶ as part of the Water Supply Project (WSP) – Eastern and Midlands Region. The objective of the study was to examine a range of population projections out to 2050, to be used as the basis for the estimation of water demand for the WSP. The study sets out regional population projections for the Planning Regions and the State, to 2050. Census 2011 provided data for the base year 2011, with projections for the years 2021, 2026, 2031, 2041, 2046 and 2050. Years 2031 and 2046 respectively, represent the furthest dates used for the CSO Regional and State Population Projections.

As a result of the WSP 'Demographic Study' and the CSO projections outlined above, the proposed growth scenarios with respect to the Greater Dublin Drainage project have been reviewed.

The WSP 'Demographic Study' outlines a range of assumption scenarios in order to assess future residential population. These scenarios are presented below;

- *A Planned Growth Scenario, providing for both 'High' and 'Low' population variations;*
- *A Most Likely Growth Scenario;*
- *A Minimum Expected Economic Growth Scenario;*
- *A Maximum Expected Economic Growth Scenario, providing for both 'High' and 'Low' population variations.*

⁶ Water Supply Project Dublin Region: Summary of Demographic Projections, AOS Planning, June 2014

As emphasised in the WSP ‘Demographic Study’ the strategic approach adopted was to utilise the available CSO 2011 census population projection documentation as the ‘spinal structure’ upon which all projections were based. Specifically, the CSO Regional Population Projections to 2031 served to inform all projections up to and including that year. All subsequent date projections utilised the State projections to 2046.

As a result, the assumption scenarios contained in the WSP ‘Demographic Study’ also align with the assumptions scenarios presented in the CSO Projections as outlined below.

1. *Planned Growth Scenario ‘Low’ - (CSO ‘M2F2 Recent’ Scenario)*
2. *Planned Growth Scenario ‘High’ - (CSO ‘M2F2 Traditional’ Scenario)*
3. *A Most Likely Growth Scenario - (CSO ‘M2F2 Modified’ Scenario)*
4. *A Minimum Expected Economic Growth Scenario – (CSO ‘M3F2’ Scenario)*
5. *A Maximum Expected Economic Growth Scenario, providing for both ‘High’ and ‘Low’ population variations – (Combinations of CSO ‘M2F1’ and ‘M1F2’ Scenarios)*

Examination of the underlying characteristics and assumptions of each of the above growth scenarios determined that scenarios 4 and 5 were unlikely to be reasonable growth scenarios to plan the major strategic infrastructure required for the GDD and were therefore removed from further consideration.

Three future population growth scenarios were therefore selected for the Greater Dublin Drainage project. These growth scenarios are outlined below and in Table 3.4.

- *Growth Scenario 1 – Planned ‘Low’ - (CSO ‘M2F2 Recent’ Scenario)*
- *Growth Scenario 2- Planned ‘High’ - (CSO ‘M2F2 Traditional’ Scenario)*
- *Growth Scenario 3 - ‘Most Likely’ - (CSO ‘M2F2 Modified’ Scenario)*

Table 3.4 Proposed Residential Population Growth Scenarios

Growth Scenario	Time Period	Annual Average Growth Rate
Growth Scenario 1 – Planned ‘Low’.	2016- 2021	Adopt the annual average growth rates derived from the ‘M2F2 Recent’ 2016 - 2021 population figures as set out in the CSO Regional Population Projections 2016 - 2031.
	2021- 2031	Adopt the annual average growth rates derived from the ‘M2F2 Recent’ 2021 - 2031 population figures as set out in the CSO Regional Population Projections 2016 - 2031.
	2031- 2041	Adopt the annual average growth rates derived from the ‘Growth Scenario 1 – Low’ 2031 - 2041 population figures as set out in the WSP ‘Demographic Study’ 2014.
	2041- 2050	Adopt the annual average growth rates derived from the ‘Growth Scenario 1 – Low’ 2041 - 2050 population figures as set out in the WSP ‘Demographic Study’ 2014.

Growth Scenario	Time Period	Annual Average Growth Rate
Growth Scenario 2 – Planned ‘High’.	2016- 2021	Adopt the annual average growth rates derived from the ‘M2F2 – Traditional’ 2016 - 2021 population figures as set out in the CSO Regional Population Projections 2016 - 2031.
	2021- 2031	Adopt the annual average growth rates derived from the ‘M2F2 Traditional’ 2021 - 2031 population figures as set out in the CSO Regional Population Projections 2016 - 2031.
	2031- 2041	Adopt the annual average growth rates derived from the ‘Growth Scenario 2 – High’ 2031 - 2041 population figures as set out in the WSP ‘Demographic Study’ 2014.
	2041- 2050	Adopt the annual average growth rates derived from the ‘Growth Scenario 2 – High’ 2041 - 2050 population figures as set out in the WSP ‘Demographic Study’ 2014.
Growth Scenario 3 – Most Likely	2016- 2021	Adopt the annual average growth rates derived from the ‘M2F2 Modified’ 2016 - 2021 population figures as set out in the CSO Regional Population Projections 2016-2031.
	2021- 2031	Adopt the annual average growth rates derived from the ‘M2F2 Modified’ 2021 - 2031 population figures as set out in the CSO Regional Population Projections 2016-2031.
	2031- 2041	Adopt the annual average growth rates derived from the ‘Growth Scenario 3 – Most Likely’ 2031 - 2041 population figures as set out in the WSP ‘Demographic Study’ 2014.
	2041- 2050	Adopt the annual average growth rates derived from the ‘Growth Scenario 3 – Most Likely’ 2041 - 2050 population figures as set out in the WSP ‘Demographic Study’ 2014.

Annual average growth rates, derived from CSO Regional Population Projections and the WSP ‘Demographic Study’ population projections, have been examined for each of the Catchment Areas. These growth rates have been adopted for the greater Dublin Drainage Project.

Table 3.5 summarises the population growth rates adopted.

Table 3.5 Proposed Residential Population Growth Rates (percentages)

Catchment Area	2016 - 2021			Post 2021-2031			Post 2031-2041			Post 2041-2050		
	Growth Scenario			Growth Scenario			Growth Scenario			Growth Scenario		
	1	2	3	1	2	3	1	2	3	1	2	3
Ringsend WwTP	0.75	0.85	0.96	0.79	1.02	1.09	0.70	0.84	0.84	0.66	0.56	0.81
9C Sewer												
Blanchards-town	0.75	0.85	0.96	0.79	1.02	1.09	0.70	0.84	0.84	0.66	0.56	0.81
Ashbourne / Ratoath	0.99	1.17	0.99	0.90	1.23	0.90	0.70	0.84	0.84	0.66	0.56	0.81
Dunboyne / Clonee	0.99	1.17	0.99	0.90	1.23	0.90	0.70	0.84	0.84	0.66	0.56	0.81
North Fringe Sewer												
Fingal 'South Fringe'	0.75	0.85	0.96	0.79	1.02	1.09	0.70	0.84	0.84	0.66	0.56	0.81
Dublin North City	0.75	0.85	0.96	0.79	1.02	1.09	0.70	0.84	0.84	0.66	0.56	0.81
NDDS Sewer												
Dublin North City	0.75	0.85	0.96	0.79	1.02	1.09	0.70	0.84	0.84	0.66	0.56	0.81
Fingal - Howth/Sutton	0.75	0.85	0.96	0.79	1.02	1.09	0.70	0.84	0.84	0.66	0.56	0.81
Other Catchments												
Swords WwTP	0.75	0.85	0.96	0.79	1.02	1.09	0.70	0.84	0.84	0.66	0.56	0.81
Malahide WwTP	0.75	0.85	0.96	0.79	1.02	1.09	0.70	0.84	0.84	0.66	0.56	0.81
Leixlip WwTP	0.75	0.85	0.96	0.79	1.02	1.09	0.70	0.84	0.84	0.66	0.56	0.81
Osberstown WwTP	0.75	0.85	0.96	0.79	1.02	1.09	0.70	0.84	0.84	0.66	0.56	0.81
9B Sewer	0.75	0.85	0.96	0.79	1.02	1.09	0.70	0.84	0.84	0.66	0.56	0.81

It is to be noted that the residential population growth rates proposed above are lower than the long term average annual growth rates for the GDA indicated in Table 3.3.

4. Existing and Future Industrial, Commercial & Institutional Load

4.1 Industrial Loadings

Industrial discharges are licensed under either the Integrated Pollution Prevention Control (IPPC) License with the EPA as the competent authority or Waste License (Section 16) issued by the Local Authority / Irish Water.

4.1.1 Existing Industrial Loadings

Information on existing industrial discharges has been obtained from a review of the 2016 AER (Annual Environmental Reports) for the various industries.

4.1.2 Future Industrial Loadings

The basis for provision for industry within the assessment of required design capacity is as follows:

- Existing industrial load to remain unchanged out to the 2050 Design Year; and
- In accordance with Irish Water's Water Service Strategic Plan (WSSP) a headroom allowance of 20% of the sum of the residential and commercial loads is provided in the design, from which capacity can be made available for future industrial loads.

4.2 Commercial & Institutional Loadings

The existing load contribution from commercial and institutional sources, (e.g. shops, offices, schools, etc) has been estimated as follows:

- Where the complete catchment to a wastewater treatment plant is being considered the commercial and institutional load contribution has been estimated by deducting the known population and industrial loadings from the measured BOD load at the treatment plant.
- Where individual sub-catchments are being considered the commercial and institutional load contribution has been assumed as 16% of the population load. This relationship has been used extensively in the estimation of flow and load for design purposes and is widely accepted at a local and national level in Ireland (source: National Urban Waste Water Study, DEHLG, vol 2, part A, section 5, 2004).

4.2.1 Future Commercial & Institutional Loadings

Future commercial & institutional loadings are assumed to grow in line with population growth.

5. Analysis of Ringsend Catchment

5.1 Existing Load on Ringsend WwTP

The organic loading, expressed in terms of population equivalent (PE) on Ringsend WwTP as of 2016 was approximately **1.808** million. This was the average day loading for 2016 and it is noted that daily load measurements at Ringsend WwTP show wide fluctuations and variability⁷.

5.2 Residential Population in Ringsend Catchment

5.2.1 Existing Population

The residential population contributing to the Ringsend WwTP in 2016 is calculated at 1,160,553 persons.

5.2.2 Future Population

A summary of the projected future residential population in the Ringsend WwTP catchment at years 2025, 2031, 2040 and 2050 for the three growth scenarios is shown in Table 5.1.

Table 5.1 Projected Future Residential Population in Ringsend WwTP catchment

Growth Scenario	Base Year		Design Year		
	2016	2025	2031	2040	2050
Scenario 1 - Planned Low	1,164,859	1,243,254	1,303,361	1,387,810	1,482,763
Scenario 2 - Planned High	1,164,859	1,260,881	1,340,041	1,444,820	1,532,053
Scenario 3 - 'Most Likely'	1,164,859	1,271,290	1,356,731	1,462,814	1,586,188

5.3 Existing Industrial Loadings in the Ringsend WwTP

The licensed industrial load expressed as a population equivalent (PE) in the Ringsend WwTP catchment is estimated as 1,038,300 PE.

However, the actual utilised industrial loading in 2016 from IPPC licenced facilities discharging to the Ringsend catchment was 230,026 PE as determined from the 2016 AER returns.

The utilised industrial loading from Section 16 licenced facilities discharging to the Ringsend catchment in 2016 is estimated at 105,632 PE

The total 2016 industrial loading discharging to the Ringsend catchment is therefore estimated at 335,658 PE

⁷ See Table 3.8 of "Ringsend Wastewater Treatment Works – Design Review Report, June 2010" for details of the significant loading variability at Ringsend WwTP.

5.3.1 Future Industrial Loadings in the Ringsend WwTP catchment

Provision for future industry within Ringsend catchment is as follows:

- Existing industrial load to remain unchanged at 335,658 PE out to the 2050 Design Year.
- An allowance of 150,000 PE is added from 2019 to provide for expansion plans of a specific Significant Industrial Customer (SIC), identified in confidential briefings to Irish Water, and.
- In accordance with Irish Water’s Water Service Strategic Plan (WSSP) a headroom allowance of 20% of the sum of the residential and commercial loads is provided in the design, from which capacity can be made available for future industrial loads.

5.4 Existing Commercial and Institutional Loading in the Ringsend WwTP

The load contribution from commercial and institutional sources in the Ringsend WwTP catchment have been estimated by deducting known residential and industrial contributions from the total load measured at the treatment plant. In this manner the commercial and institutional load contribution to Ringsend WwTP in 2016 has been estimated at **311,835 PE** or c.27% of residential contribution.

5.4.1 Future Commercial and Institutional Loading in the Ringsend WwTP

Future commercial & institutional loadings in the Ringsend WwTP catchment are assumed to grow in line with population growth.

5.5 Projected Treatment Capacity Requirements at Ringsend

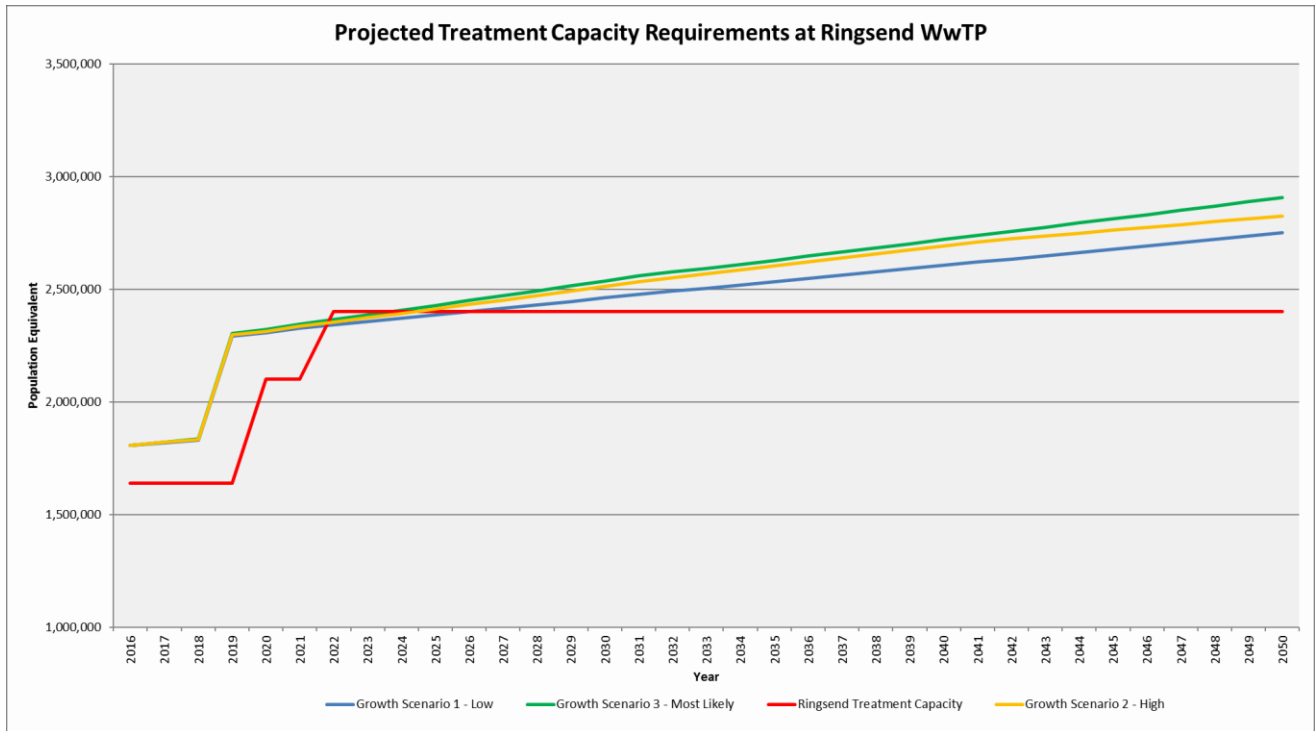
On completion of the current upgrade of Ringsend WwTP the works will have an installed treatment capacity of 2.4 million PE and further expansion beyond this level is not considered feasible.

The projected average day population equivalent, i.e. the combination of residential, commercial and industrial projections, treatment capacity requirements for the Ringsend WwTP Catchment under the three growth scenarios examined is summarised in Table 5.2 and illustrated in Figure 5.1. The treatment capacity of Ringsend WwTP of 2.4 million PE average day load is also shown. A detailed analysis of the projected population equivalent loadings is provided in Appendix B.

Table 5.2 Summary of Projected Treatment Capacity Requirements – Ringsend WwTP Catchment

Growth Scenario	Base Year		Design Year		
	2016	2025	2031	2040	2050
Scenario 1 –Planned Growth - Low	1,808,046	2,385,931	2,477,439	2,606,007	2,750,567
Scenario 2– Planned Growth - High	1,808,046	2,412,766	2,533,282	2,692,801	2,825,608
Scenario 3 - Most Likely Growth	1,808,046	2,428,613	2,558,691	2,720,196	2,908,024

Fig. 5.1 Projected Treatment Capacity Requirements – Ringsend Catchment



Under Growth Scenario 1 – ‘Planned Growth – Low’, the projected treatment capacity requirements exceed the ultimate installed treatment capacity of 2.4 million PE from year 2027. At 2050 this exceedance is projected at 350,567 PE.

Under Growth Scenario 2 – ‘Planned Growth – High’, the projected treatment capacity requirements exceed the ultimate installed treatment capacity of 2.4 million PE from year 2025. At 2050 this exceedance is projected at 425,608 PE.

Under Growth Scenario 3– ‘Most Likely Growth’ the projected treatment capacity requirements exceed the ultimate installed treatment capacity of 2.4 million PE from year 2024. At 2050 this exceedance is projected at 508,024 PE.

5.6 Appraisal of Ringsend WwTP’s treatment capacity

As shown above, the projected treatment capacity requirements in the Ringsend catchment will exceed the ultimate installed treatment capacity of 2.4 million PE between 2024 and 2027 depending on the actual growth rate realised in the catchment. The deficit in treatment capacity requirements in the Ringsend catchment is projected to range between 350,567 PE and 508,024 PE at 2050.

5.7 Conclusions

The constraints on the future expansion of Ringsend WwTP beyond its ultimate capacity as originally identified by GDSDS remain relevant. Furthermore, the capacity constraints in the sewer network serving the Ringsend WwTP catchment, as identified by GDSDS, also remain relevant. Therefore, future development within this catchment cannot be catered for by Ringsend WwTP alone and additional treatment capacity is required in the catchment.

The GDD project, encompassing a new Regional WwTP, marine outfall pipe and orbital drainage system will provide the additional wastewater treatment capacity required in the catchment. By diverting flow and load out of the Ringsend catchment to the new Regional WwTP the GDD project will free up capacity at Ringsend WwTP and in its contributing network. The GDD project will therefore enable growth to continue out to 2050 in the sub-catchments that are diverted to the new Regional WwTP and also in the other sub-catchments that remain served by RingsendWwTP.

6. Analysis of Existing Regional WwTPs

6.1 Introduction

This chapter assesses the projected treatment capacity requirements in four drainage catchments served by existing Regional WwTPs that were identified by GSDSDS as potentially having an influence on the future required treatment capacity of the proposed new Regional WwTP. The four catchments assessed are Malahide, Swords, Lower Liffey Valley (Leixlip WwTP) and the Upper Liffey Valley (Osberstown WwTP). A summary discussion of the assessment is provided hereunder with the full tabular analysis provided in Appendix A.

6.2 Malahide WwTP

6.2.1 Existing Organic Load

The average daily organic load on Malahide WwTP in 2016, expressed as population equivalents was 16,692 PE. The existing treatment capacity of the plant is 21,000 PE.

6.2.2 Growth in the Malahide WwTP Catchment

The projected treatment capacity requirements under the three growth scenarios examined are summarised hereunder.

Growth Scenario 1 – Planned Growth - Low

Under this growth scenario, the 21,000 PE capacity at Malahide WwTP is exceeded in year 2048. By year 2050 the required treatment capacity in this catchment is projected at 21,326 PE.

Growth Scenario 2– Planned Growth - High

Under this growth scenario, the 21,000 PE capacity at Malahide WwTP is exceeded in year 2042. By year 2050 the required treatment capacity in this catchment is projected at 22,035 PE.

Growth Scenario 3 – Most Likely Growth

Under growth scenario 3, the 21,000 PE capacity at Malahide WwTP is exceeded in year 2040. By year 2050 the required treatment capacity in this catchment is projected at 22,814 PE.

6.2.3 Impact of Future Growth

The location of Malahide WwTP precludes the possibility of physically expanding the plant to provide the projected future treatment capacity requirements.

The WwTP currently discharges to the environmentally sensitive water of the outer Broadmeadow Estuary, which places further constraints on the possibility of expanding the plant at its current location.

Options to provide for future projected treatment capacity requirements in this catchment include

- Diversion of flow from part of the catchment to the Swords WwTP catchment (short term solution);
- Diversion of flow from part of the catchment to the North Fringe Sewer for treatment at Ringsend WwTP (short term solution);
- Diversion of flow from part of the catchment to the new Regional WwTP (long term solution)

Diversion of flow from the Malahide catchment to either the Swords catchment or to the North Fringe sewer would only be temporary short term options. The recommended long term solution for the Malahide catchment is to divert flow from part of the catchment to the proposed Regional WwTP.

6.3 Swords WwTP

6.3.1 Existing Population

The average daily organic load on Swords WwTP in 2016, expressed as population equivalents was 58,341 PE. The existing treatment capacity of the plant is 90,000 PE.

6.3.2 Growth in the Swords Catchment

The projected treatment capacity requirements under the three growth scenarios examined are summarised hereunder.

Growth Scenario 1 – Planned Growth - Low

Under this growth scenario, the 90,000 PE capacity at Swords WwTP is not exceeded prior to 2050

Growth Scenario 2– Planned Growth - High

Under this growth scenario, the 90,000 PE capacity at Swords WwTP is exceeded in year 2048. By year 2050 the required treatment capacity in this catchment is projected at 91,228 PE.

Growth Scenario 3 – Most Likely Growth

Under growth scenario 3, the 90,000 PE capacity at Swords WwTP is exceeded in year 2045. By year 2050 the required treatment capacity in this catchment is projected at 94,380 PE.

6.3.3 Impact of Future Growth

Swords is one of Ireland's fastest growing towns. It is also worth noting that Swords has been identified as a Metropolitan Consolidation Town within the RPG Settlement Typology and Hierarchy for the GDA. It is therefore expected that long term growth could see Swords expanding up to 100,000 in a planned and phased manner (Source RPG 2010 & Fingal County Development Plan 2017 – 2023). This implies that Swords, given its current population, is likely to be the main growth area within the Metropolitan Area of Fingal for the foreseeable future.

The future treatment capacity requirements in this catchment are projected to exceed the installed capacity at Swords WwTP from 2045 onwards depending on actual growth realised. The long term solution, as recommended by GSDS, to anticipated treatment capacity deficits in this catchment

post 2050 is to divert flow from part of the catchment to the proposed Regional WwTP. This solution could be developed in conjunction with flow diversions from the Malahide catchment.

6.4 Lower Liffey Valley (Leixlip WwTP) Catchment

6.4.1 Existing Population

The average daily organic load on Leixlip WwTP in 2016, expressed as population equivalents was 126,000 PE. The existing treatment capacity of the plant is 150,000 PE.

6.4.2 Growth in the Leixlip Catchment

The projected treatment capacity requirements in the Lower Liffey Valley catchment under the three growth scenarios examined are summarised hereunder. It should be noted that the projected growth in the Lower Liffey Valley catchment includes for the expansion plans of a specific Significant Industrial Customer (SIC), identified in confidential briefings to Irish Water.

Growth Scenario 1 – Planned Growth - Low

Under this growth scenario the 150,000 PE upgraded capacity at Leixlip WwTP is exceeded in year 2019. By year 2050 the required treatment capacity in this catchment is projected at 195,499 PE..

Growth Scenario 2 – Planned Growth - High

Under this growth scenario the 150,000 PE upgraded capacity at Leixlip WwTP is exceeded in year 2019. By year 2050 the required treatment capacity in this catchment is projected at 264,216 PE.

Growth Scenario 3 Most Likely Growth

Under this growth scenario the 150,000 PE upgraded capacity at Leixlip WwTP is exceeded in year 2019. By year 2050 the required treatment capacity in this catchment is projected at 267,969 PE.

6.4.3 Impact of Future Growth

The expansion of this catchment beyond 150,000 PE cannot be served by the Leixlip WwTP alone due to the limiting assimilative capacity of the receiving water (River Liffey) under low flow conditions⁸. GDSDS recommended transferring flow and load in excess of the 150,000 PE capacity of Leixlip WwTP to the proposed Regional WwTP. Irish Water advise that works to transfer the excess flow and load from Leixlip WwTP are currently at planning stage. These works would initially transfer the flows to the Blanchardstown catchment for onward transfer to Ringsend WwTP for treatment in the short term. Under the GDD project these flows would be diverted to the Regional WwTP with the flows from the Blanchardstown catchment.

⁸ Leixlip WwTW Environmental Impact Statement – Effluent Quality for Leixlip WwTW

6.5 Upper Liffey Valley (Osberstown WwTP) Catchment

6.5.1 Existing Population

The average daily organic load on Osberstown WwTP in 2016, expressed as population equivalents was 80,239 PE. The existing treatment capacity of the plant is 130,000 PE.

6.5.2 Growth in Osberstown Catchment

Under all three growth scenarios, the 1300,000 PE treatment capacity at Osberstown WwTP is not exceeded prior to 2050

6.5.3 Impact of Future Growth

It is worth noting as part of the review of the County Development Plan Kildare County Council conducted a survey which indicated that residential and mixed-use zoned lands in the Upper Liffey Valley Catchment were in excess of 500ha. Significant development in this catchment could lead to treatment capacity requirements far in excess of the installed capacity of 130,000 PE.

Development in this catchment, which would require treatment capacity to be provided beyond the installed capacity of 130,000 PE will require further studies to determine the optimum manner in which such development could be catered for.

6.6 Summary

The impact of projected future growth in the catchments of these regional plants indicates that there will be significant challenges facing these catchments in the future. It may not prove to be technically feasible or economically sustainable to overcome these challenges by further upgrade works at the individual treatment plants and alternative solutions may have to be developed.

Potential solutions for each of the four regional WwTPs examined here are summarised below:

- Projected growth in the Lower Liffey Valley catchment requires that flow and load in excess of the installed 150,000 PE treatment capacity at Leixlip WwTP be diverted to the Regional WwTP as soon as this plant is commissioned. Irish Water advise that works to transfer the excess flow and load from Leixlip WwTP are currently at planning stage.
- Projected growth in the Swords and Malahide catchments requires that flow and load in excess of the installed treatment capacities be diverted to the Regional WwTP post 2050.
- Whilst the 130,000 PE treatment capacity at Osberstown WwTP is not exceeded in any of the three growth scenarios examined prior to 2050 it is possible, given the zoned lands available in this catchment, that the actual growth realised may exceed projections. Should this be the case further studies are recommended to ascertain the optimum solution to provide potential future treatment capacity requirements in this catchment.

6.7 Conclusions

Of the four existing regional WwTPs assessed in this chapter only the Lower Liffey Valley (Leixlip WwTP) will impact on the treatment capacity to be provided at the proposed Regional WwTP prior to 2050. Works to transfer excess flow and load from Leixlip WwTP to the Blanchardstown (9C Sewer) catchment are currently at planning stage. These excess flows from Leixlip WwTP have been considered in the assessment of the Ringsend catchment discussed in Chapter 5. Therefore, the additional treatment capacity required ranges between 350,567 PE and 508,024 PE to provide for growth in the Ringsend catchment out to 2050. This additional treatment capacity cannot be provided by further development of Ringsend WwTP beyond that which is already in planning. Therefore, a new wastewater treatment plant is required. The GDD project will provide the required additional treatment capacity.

In our view, the statutory requirement that Irish Water should be in a position to address in its strategic planning, '*existing and reasonably foreseeable deficiencies in the provision of water services*' requires a particular and very important perspective on the use of Planning Scenarios in infrastructural planning for assets of long working life.

Of the three growth scenarios examined the 'most likely' scenario (or growth scenario 3) sets out the treatment capacity requirement profile of greatest probability given what is known at the present time. Therefore, it is prudent for planning purposes to develop the GDD on the basis of this growth scenario. It is therefore recommended that the GDD project be designed to provide a wastewater treatment capacity of 500,000 PE.

7. Analysis of Northern & Western Sub-Catchments

7.1 Introduction

As recommended in Chapter 6 the GDD project will provide an additional 500,000 PE of treatment capacity, primarily to augment the existing treatment capacities provided at Ringsend WwTP and Leixlip WwTP. To mobilise this capacity, it is necessary to divert flow and load out of these catchments to the proposed Regional WwTP in order to keep the average daily organic loads on these plants below their installed treatment capacities.

As noted previously, Irish Water currently have work at planning stage to transfer excess flows away from Leixlip WwTP. In the short term, it is proposed to transfer these flows to the 9C sewer in Blanchardstown for onward transfer to the Ringsend WwTP. Ultimately, under the GDD project these flows will transfer with the flows from Blanchardstown to the proposed Regional WwTP.

The transfer of flows out of the Ringsend catchment revolve around the northern, north-western and western sub-catchments of the current Ringsend catchment. These sub-catchments comprise of,

- *The Blanchardstown (9C Sewer) sub-catchment of Ringsend WwTP (includes the Meath towns & villages of Ashbourne, Ratoath, Kilbride, Dunboyne & Clonee),*
- *The North Dublin (North Fringe Sewer & North Dublin Drainage Scheme (NDDS) Sewer) sub-catchment of Ringsend WwTP,*
- *The South Dublin – Lucan/Clondalkin (9B Sewer) sub-catchment of Ringsend WwTP*

This chapter examines the projected treatment capacity requirements for each of the above sub-catchments out to the design year horizon of 2050 to establish the optimum implementation strategy for the required flow diversions.

7.2 Analysis of Projected Treatment Capacity Requirements

7.2.1 Blanchardstown (9C Sewer) Sub-Catchment

The projected treatment capacity required, expressed as population equivalent, in the 9C Sewer Sub-Catchment north of the M50, under the ‘most likely’ growth scenario is summarised in Table 7.1. This includes population growth as per Table 3.5, commercial growth as per Section 4.2, provision for industry as per Section 4.1 and the proposed load transfer from the Leixlip WwTP catchment.

Table 7.1 Summary of Projected Treatment Capacity Requirements (PE) – 9C Sewer Sub-Catchment

	Design Year				
	2016	2025	2031	2040	2050
Scenario 3 – Most Likely Growth	166,040	363,385	376,151	392,697	411,939

7.2.2 North Dublin (North Fringe Sewer) Sub-Catchment

The projected treatment capacity required, expressed as population equivalent, in the North Fringe Sewer Sub-Catchment, under the ‘most likely’ growth scenario is summarised in Table 7.2. This includes population growth as per Table 3.5, commercial growth as per Section 4.2, and provision for industry as per Section 4.1.

Table 7.2 Summary of Projected Treatment Capacity Requirements (PE) – NFS Sub-Catchment

Growth Scenario	Design Year				
	2016	2025	2031	2040	2050
Scenario 3– Most Likely Growth	135,463	180,233	191,072	204,172	219,406

7.2.3 North Dublin (NDDS Sewer) Sub-Catchment

The projected treatment capacity required, expressed as population equivalent, in the NDDS Sewer Sub-Catchment, under the ‘most likely’ growth scenario is summarised in Table 7.3. This includes population growth as per Table 3.5, commercial growth as per Section 4.2, and provision for industry as per Section 4.1.

Table 7.3 Summary of Projected Treatment Capacity Requirements (PE) – NDDS Sub-Catchment

Growth Scenario	Design Year				
	2016	2025	2031	2040	2050
Scenario 3 – Most Likely Growth	220,078	284,087	302,068	324,393	350,356

7.2.4 South Dublin – Lucan/Clondalkin (9B Sewer) Sub-Catchment

The projected treatment capacity required, expressed as population equivalent, in the 9B Sewer Sub-Catchment, under the ‘most likely’ growth scenario is summarised in Table 7.4. This includes population growth as per Table 3.5, commercial growth as per Section 4.2, and provision for industry as per Section 4.1.

Table 7.4 Summary of Projected Treatment Capacity Requirements (PE) – 9B Sewer Sub-Catchment

Growth Scenario	Design Year				
	2016	2025	2031	2040	2050
Scenario 3 – Most Likely Growth	85,255	111,628	119,036	128,234	138,931

7.3 Discussion on Projected Treatment Capacity Requirements

It is evident from the above analysis that no one sub-catchment can provide the required 500,000 PE capacity on its own. Flow diversions from at least two catchments are therefore required. As the proposed Regional WwTP is located in north county Dublin the two catchments given prime

consideration for diversion are the Blanchardstown (9C Sewer) catchment and the North Fringe Sewer (NFS) catchment.

7.3.1 Blanchardstown (9C Sewer) Catchment

The largest flow and load diversion is available from the Blanchardstown catchment. This flow and load is inclusive of the transferred flows from Leixlip. Constraints on the 9C sewer network downstream of Blanchardstown, which severely limit its capacity to transfer the projected future flows in this catchment to Ringsend WwTP include:

- The 9C sewer twin syphons under the River Liffey;
- The Davitt Road/Dolphin Road Sewer, which also serves the Lucan/Clondalkin 9B sewer; and
- the inlet syphon from the Main Lift Pumping Station (MLPS) to Ringsend WwTP.

Diversion of this sub-catchment to the proposed Regional WwTP would therefore free up capacity in the downstream network, in particular the Davitt Road/Dolphin Road sewer and the Grand Canal Tunnel Sewer (GCTS), enabling growth to continue in the connected catchments. This applies in particular to the Lucan/Clondalkin (9B sewer) catchment where there is substantial scope for development for housing (Adamstown SDZ and Clonburris SDZ) and industrial units, and the City Centre catchment where growth through urban regeneration, development of brownfield sites and urban densification is occurring.

It is therefore recommended that diversion of this sub-catchment to the proposed Regional WwTP be considered a priority. This diversion will provide a projected 411,939 PE of the required 500,000 PE at 2050 under the 'most likely' growth scenario.

7.3.2 North Dublin (North Fringe Sewer) Catchment

A diversion of the entire North Fringe Sewer (NFS) catchment would provide a projected treatment capacity requirement of 219,406 PE at 2050 under the 'most likely' growth scenario at the proposed Regional WwTP. Coupled with the diversion of the Blanchardstown (9C Sewer) catchment this would provide a projected treatment capacity requirement of 631,345 PE at 2050 at the proposed Regional WwTP. This exceeds the required 500,000 PE load diversion therefore a full diversion of this catchment is not required before 2050. A partial diversion of the NFS, as discussed below, is instead proposed.

This partial diversion of the NFS catchment envisages intercepting the NFS between existing NFS manholes MH1-50 and MH1-51, which are located approximately 500m due south of the proposed Regional WwTP and immediately north of the R139. The diversion sewer would be routed into the Regional WwTP along the proposed access road off the R139. In this scenario all sub-catchments of the NFS located to the west of the Regional WwTP will be diverted, including north Finglas, Ballymun and Dublin Airport. The projected treatment capacity requirements expressed as population equivalent, for these sub-catchments under the 'most likely' growth scenario is summarised in Table 7.5

Table 7.5 Summary of Projected Treatment Capacity Requirements (PE) – NFS Sub-Catchments west of Regional WwTP

	Design Year				
	2016	2025	2031	2040	2050
Scenario 3 – Most Likely Growth	53,108	75,036	79,089	84,703	89,973

This proposed partial diversion of the NFS catchment will provide a projected 89,973 PE treatment capacity requirement at the proposed Regional WwTP. Coupled with the proposed diversion of the Blanchardstown (9C Sewer+ Leixlip transfer) catchment the required treatment capacity at the proposed Regional WwTP is projected as 501,912 PE at 2050. This proposed partial diversion would also reduce the flows arriving at Sutton Pumping Station thereby relieving operational pressures at this facility.

The required 500,000 PE diversion of projected treatment capacity requirement from the Ringsend catchment is therefore satisfied by diverting the Blanchardstown (9C Sewer + Leixlip transfer) catchment and the sub-catchments of the NFS located west of the proposed Regional WwTP.

7.3.3 North Dublin (NDDS Sewer) Catchment

With the proposed diversions of the Blanchardstown (9C Sewer) catchment and the NFS sub-catchments west of the proposed Regional WwTP there is no requirement to divert the NDDS Sewer catchment prior to 2050.

Furthermore, it should be noted that diversion of this catchment is dependent on the diversion of the entire NFS catchment as it is not considered feasible to divert the NDDS sewer until the NFS catchment (including Portmarnock and Baldoyle) is diverted as discussed in Chapter 8.

7.3.4 South Dublin – Lucan/Clondalkin (9B Sewer) Catchment

As discussed above the diversion of the Blanchardstown (9C Sewer) sub-catchment will free up capacity in the downstream sewer network, which also serves the Lucan/Clondalkin (9B Sewer) catchment. As a result, there is no requirement to divert this catchment to the proposed Regional WwTP before 2050.

8. Options for Diverting the Northern & Western Sub-Catchments

8.1 Introduction

This chapter examines options for diverting the main trunk sewers serving the catchments in the northern, north- western and western area of the Ringsend catchment to the proposed Regional WwTP.

8.2 Blanchardstown (9C Sewer) Catchment

Irish Water have recently received planning permission (FW17A/0083) for a drainage scheme to duplicate the existing 9C sewer in the Tolka River Valley Park between the townlands of Parlickstown and Deanstown in Mulhuddart and Blanchardstown, Dublin 15.

In considering options for diverting flows from the 9C Sewer it is assumed that the works included in this planning permission will be completed in advance of or in parallel with the Greater Dublin Drainage project.

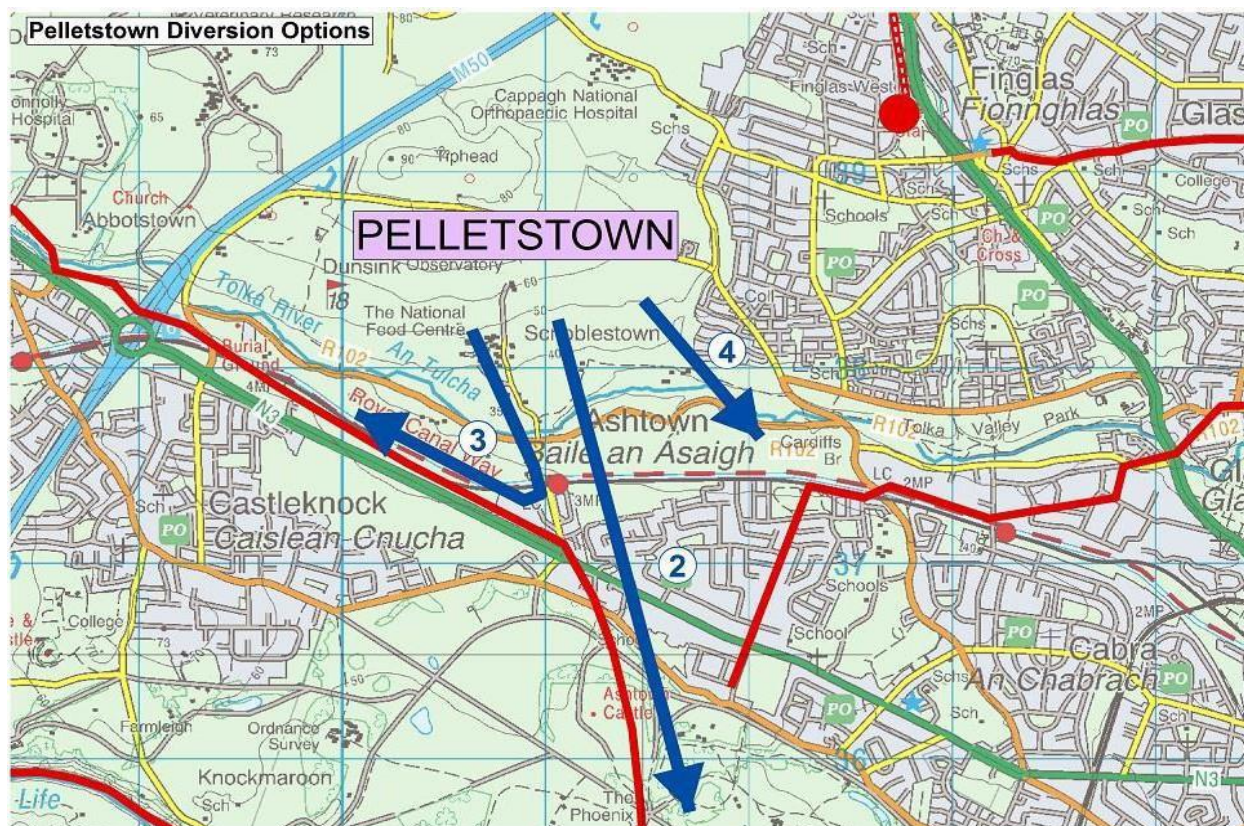
The Blanchardstown – Clonshagh Orbital Sewer will transfer flows from the existing Blanchardstown drainage catchment, which includes Blanchardstown and its environs and the Meath towns and villages of Ashbourne, Ratoath, Kilbride, Dunboyne & Clonee, to the proposed Regional WwTP at Clonshagh (Clonsgaugh). The orbital sewer commences in the grounds of Waterville Park, Blanchardstown where it intercepts the 9C sewer. From this point it is routed through the grounds of Connolly Hospital and the grounds of the National Sports Campus to the proposed Abbotstown Pumping Station, located adjacent to the M50 see Figure 8.1. From this pumping station the Orbital Sewer it is routed north of and generally parallel to the M50 to Clonshagh passing, en-route, south of the Dublin Airport complex. The lands along the length of the orbital sewer are generally open fields with agriculture the main land use pattern. The total length of this Orbital Sewer is 13,723 m.

Figure 8.1 Diversion Options for 9C Sewer to Orbital Sewer



For the residual 9C Sewer Catchment downstream of the M50 three options for dealing with the flows (ref. options 2, 3 & 4 on Fig 8.1) have been examined as set out hereunder. All options have been considered in conjunction with dealing with potential future flows from the Pelletstown area, refer to Fig. 8.2 below, which is identified as a Strategic Development and Regeneration Area (SDRA) in the Dublin City Development Plan 2016 – 2022 (Interim Publication) and therefore one of the key areas in the north city area for future development.

Figure 8.2 Foul Drainage Options for the Pelletstown SDRA



Option 2 as shown on Figure 8.1 is to leave these residual flows continue to flow through one of the Liffey Siphons to the Grand Canal Tunnel sewer. However, this residual flow is insufficient to maintain self-cleansing velocities in the Liffey siphon. This concern may be overcome by draining the Pelletstown area in this direction also (Option 2 on Figure 8.2).

Option 3, as shown on Figure 8.1, considered collecting these residual flows to a small pumping station and pump them back to the main 9C Sewer interception point. Crossing of the M50 could be achieved by using the existing pipe bridge as a pipe sleeve for the pumped main in the reverse direction. Future flows from the Pelletstown area could also be drained to the Orbital Sewer by using this pumping station (Option 3 on Figure 8.2).

Option 4 as shown on Figure 8.1 examined connecting the residual flows to the head of the NDDS sewer. This option would compete for capacity in the NDDS sewer with future flows from Pelletstown (Option 4 on Figure 8.2) and has the potential to impact unfavourably on downstream CSOs. Modelling of the NDDS sewer would be required to confirm this option.

Option 2, allowing the flows in the 9C Sewer Catchment downstream of the M50 to continue to flow to the GCTS, is the preferred option. Future flows from the Pelletstown SDRA should also be connected to the 9C Sewer once the catchment upstream of the M50 is diverted. It is recommended that the potential for diverting the Ashtown section of the NDDS sewer, south of the Railway and canal

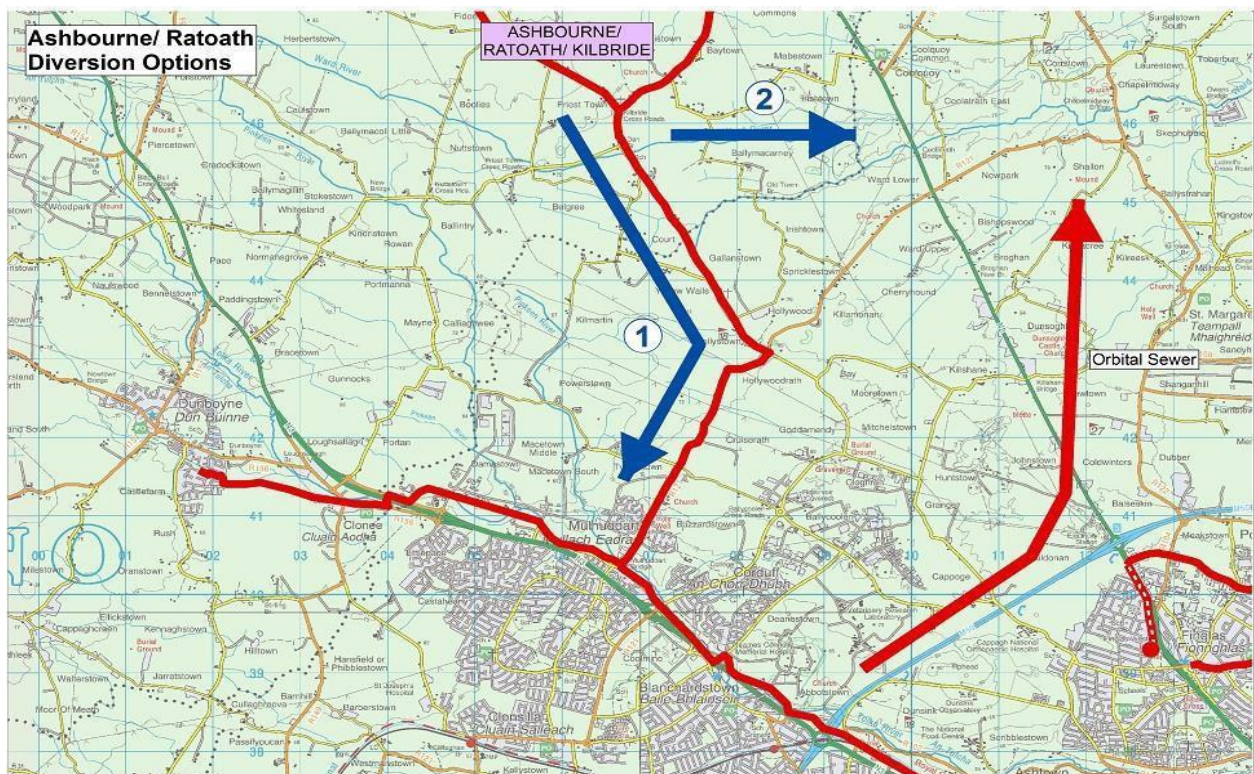
crossing, to the 9C Sewer in the vicinity of the Phoenix Park, to augment flows through the Liffey Syphons following diversion of the catchment upstream of the M50 be further examined.

Ashbourne/ Ratoath

The towns of Ashbourne and Ratoath in County Meath currently drain to the 9C Sewer via a pumping station at Kilbride. The GSDS recommendations indicated that foul flows from both these towns would be pumped directly to the Orbital Sewer from Kilbride. Modelling work undertaken on the 9C Sewer as part of the BRDS Preliminary Report also assumed that Ashbourne and Ratoath would be pumped directly to the Orbital Sewer from Kilbride and thus the future foul flows from these two towns were not considered in model runs post 2020 in examining options for upgrading the 9C Sewer.

Additional model runs, testing revised design scenarios, on the BRDS 9C Sewer model indicate that capacity exists in the 9C duplication to retain the flows pumped from Ashbourne, Ratoath and Kilbride without significant detriment in the 9C Sewer Catchment. This is the preferred option for transferring of flows and is shown as Option 1 on Figure 8.3.

Figure. 8.3 Connection Options for Ashbourne/Ratoath to Orbital Sewer



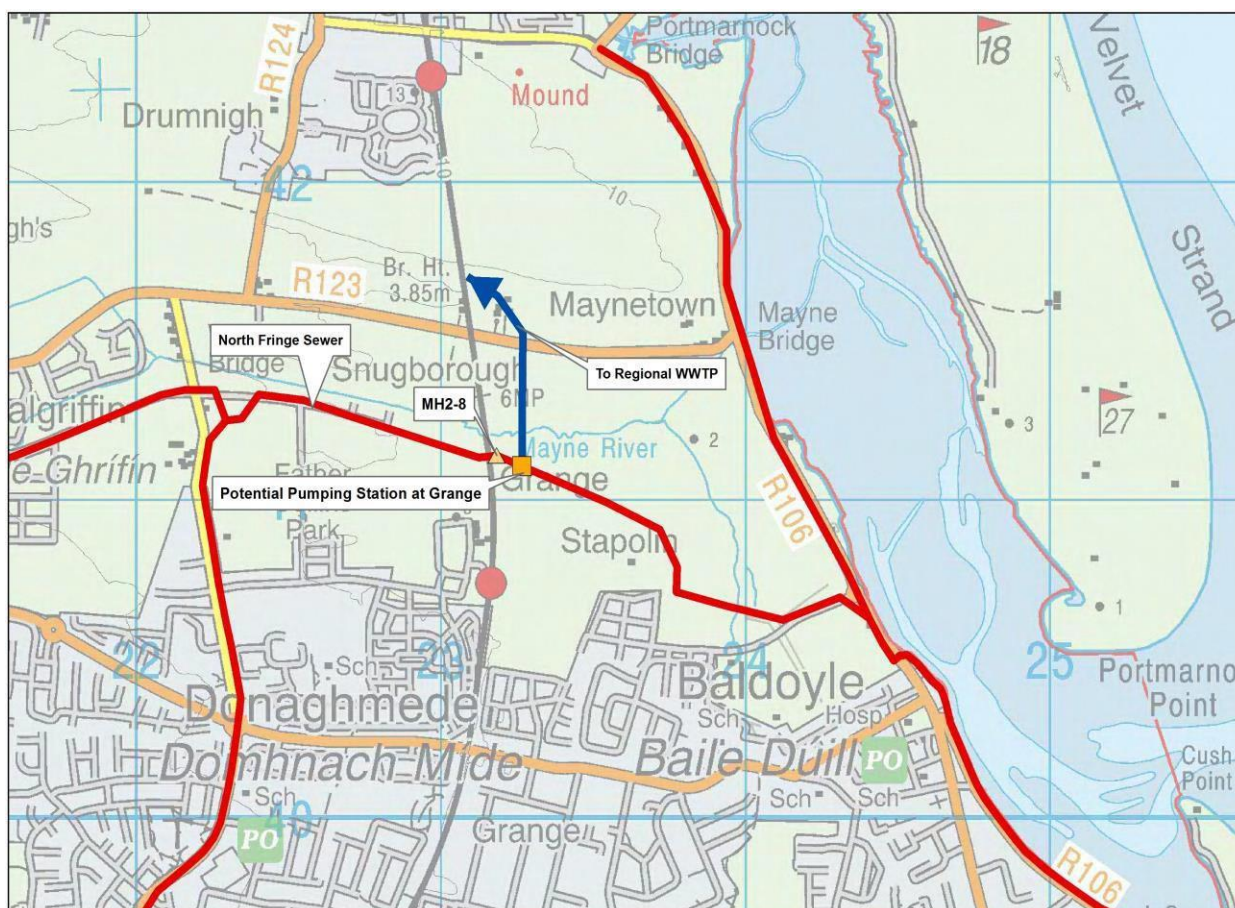
Option 1 (Figure 8.3) considers retaining the connection from Ashbourne and Ratoath to the 9C Sewer.

8.3 North Fringe Sewer (NFS) Catchment

Diversion options for this catchment considered diversion of the entire catchment and a partial diversion of the NFS sub-catchments west of the proposed Regional WwTP.

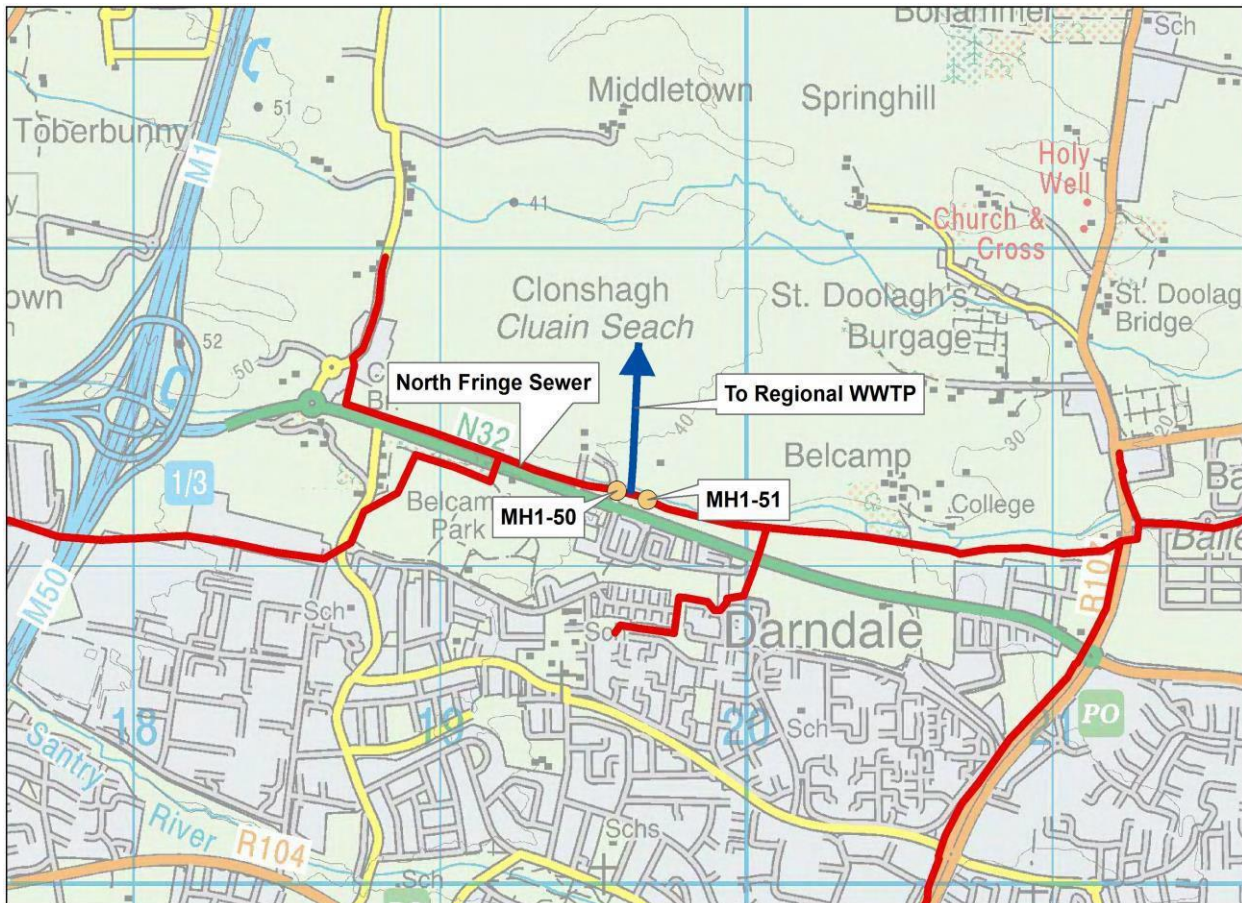
Diversion of the entire NFS catchment envisages interception of the NFS at manhole MH2-8, downstream of the Grange storm tank and immediately east of the Dublin-Belfast rail line in the townland of Grange (refer to Figure 8.4), and diverting flows to a new Grange pumping station for transfer to the proposed Regional WwTP. Flows from the existing Portmarnock and Baldoyle Pumping Stations would also be diverted to the new Grange Pumping Station at this stage.

Figure. 8.4 NFS Catchment Diversion



The partial diversion of the NFS catchment envisages interception of the NFS between manholes MH1-50 and MH1-51, which are located approximately 500m due south of the proposed Regional WwTP (refer to Figure 8.5), and diverting flows directly to the Regional WwTP. The diversion sewer would be routed into the Regional WwTP along the proposed access road off the R139. In this scenario all sub-catchments of the NFS west of the Regional WwTP would be diverted, including north Finglas, Ballymun and Dublin Airport. This option would also significantly reduce the pumping requirements at Grange Pumping Station when the entire catchment is diverted.

Figure. 8.5 Partial Diversion of the NFS Catchment



8.4 North Dublin Drainage Scheme (NDDS) Sewer Catchment

Diversion of this catchment is dependent on the diversion of the entire NFS catchment as it is not considered feasible to divert the NDDS sewer until the NFS catchment (including Portmarnock and Baldoyle) is diverted for the following reason:

- *A new pipe would have to be constructed from Sutton pumping station to the new Regional WwTP. Routing of this pipeline would be difficult as a land based route is not available and a sea route would take the pipe under the DART rail line and through the environmentally sensitive Baldoyle Estuary.*

When the NFS catchment, (including Portmarnock and Baldoyle flows) is fully diverted, as discussed in Section 8.3, the existing 1,600mm diameter pipe between the Grange Tank and Sutton Pumping Station would have no flow and could therefore be used to transfer flows from the NDDS sewer and Howth/ Sutton to a pumping station located at Grange via Sutton Pumping Station. This is illustrated as Option 2 on Figure 8.6. The 1,600mm diameter pipe may have to be lined with a suitable liner, depending on pressure analysis of this pipe, to allow it act in its new configuration as the rising main from Sutton Pumping Station to a proposed Grange Pumping Station.

Figure. 8.6 Connection Options for North Dublin Catchment to Orbital Sewer



8.5 9B (Lucan/ Clondalkin) Sub-Catchment

A strict limit of 2.0 m³/sec was placed on pass forward flows from the 9B Sewer Catchment to the Grand Canal Tunnel Sewer (GCTS) in the GSDSDS final strategy recommendations. This limit on pass forward flows was set, due to capacity constraints in the GCTS and particularly in the 9B/9C Sewer along Davitt Road / Dolphin Road, and the requirement to accommodate pass forward flows of 2.70 m³/sec from the 9C Sewer Catchment in the Davitt Road / Dolphin Road sewer and the GCTS.

The interception and diversion of 9C Sewer (Blanchardstown) flows to the Orbital sewer, now proposed, will free up capacity in the Grand Canal Tunnel Sewer (GCTS) and more particularly in the 9B/9C Davitt Road/Dolphin Road sewer. This should allow flows greater than the 2.0 m³/sec limit set by GSDSDS to be passed forward from the 9B Sewer Catchment to the GCTS.

Modelling work carried out by others as part of the BRDS Preliminary Report indicates that the pipe full capacity of the sewer along Davitt Road/Dolphin Road varies from a minimum of 3.125 m³/sec in the flattest sections of this sewer to a maximum of 6.175 m³/sec in the steeper sections.

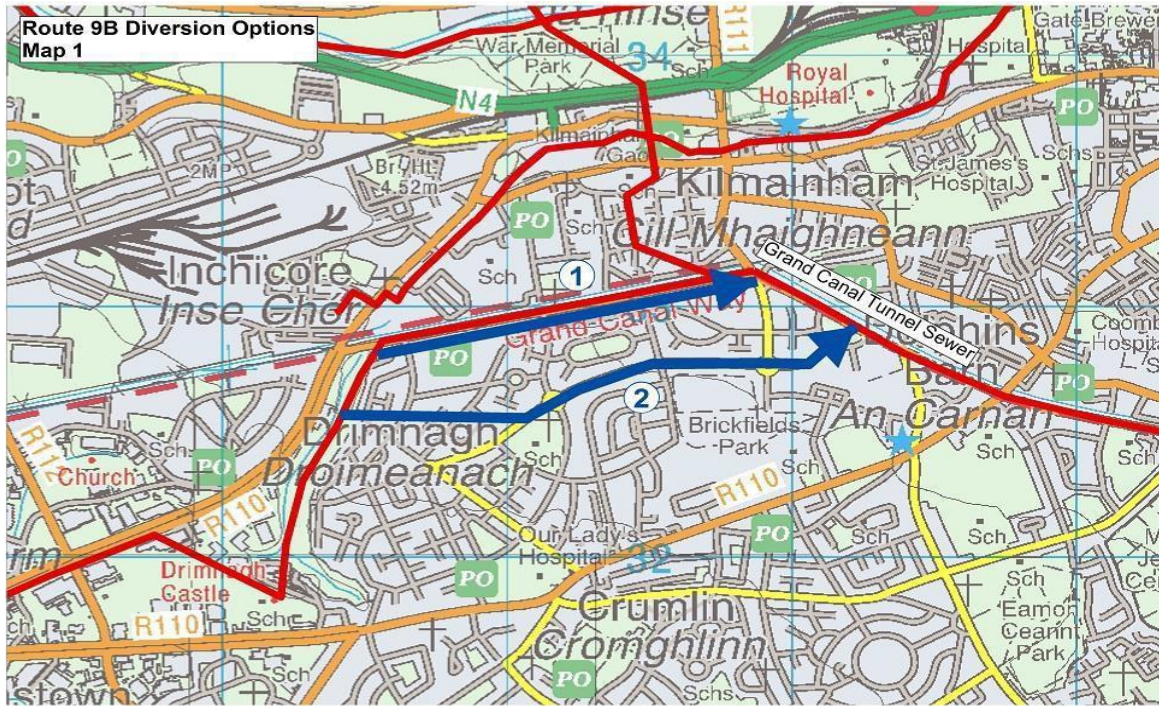
Flow measurement records for the years 2009 and 2010 indicate that flows from the 9B Sewer Catchment were significantly less than the GSDSDS limit of 2.0 m³/sec placed on pass forward flows from 9B Sewer Catchment to the GCTS.

Modelling work carried out on the 9B Sewer, as part of the GSDSDS, projected year 2031 Dry Weather Flow at 1.42 m³/sec, with a wet weather peak flow estimated as 12.03 m³/sec, based on the 100-year return period rainfall of 180-minute duration. It should be noted that significant inflow of storm flows (equivalent to runoff from 7.5% of gross future development area) to the foul sewers was allowed for in the GSDSDS models. This volume of storm inflow to the foul sewers is unsustainable as a matter of best practice design and it is recommended that the allowance for storm run-off equivalent to 7.5% of gross future development area to foul sewers be reconsidered, especially in light of the New Development Policies recommended as part of the GSDSDS.

These modelling scenarios need to be validated in terms of catchment, storm run-off management and load issues. Irish Water advise that a separate strategic study will be commissioned on the 9B catchment to examine this in detail.

Pass forward flows from the 9B Sewer Catchment to the GCTS should be maximised to make best use of existing downstream infrastructure, before consideration of flow diversion to the Orbital sewer is considered. Furthermore, a realistic view of stormwater management in the existing development and effective planning control of future developments (using SUDS systems) should be taken. Options for maximizing use of downstream infrastructure, particularly the GCTS, include duplication of the Davitt Road sewer (Option 1 on Figure 8.7) or part diversion of the pass forward flows to the Dolphin Road sewer (Option 2 on Figure 8.7).

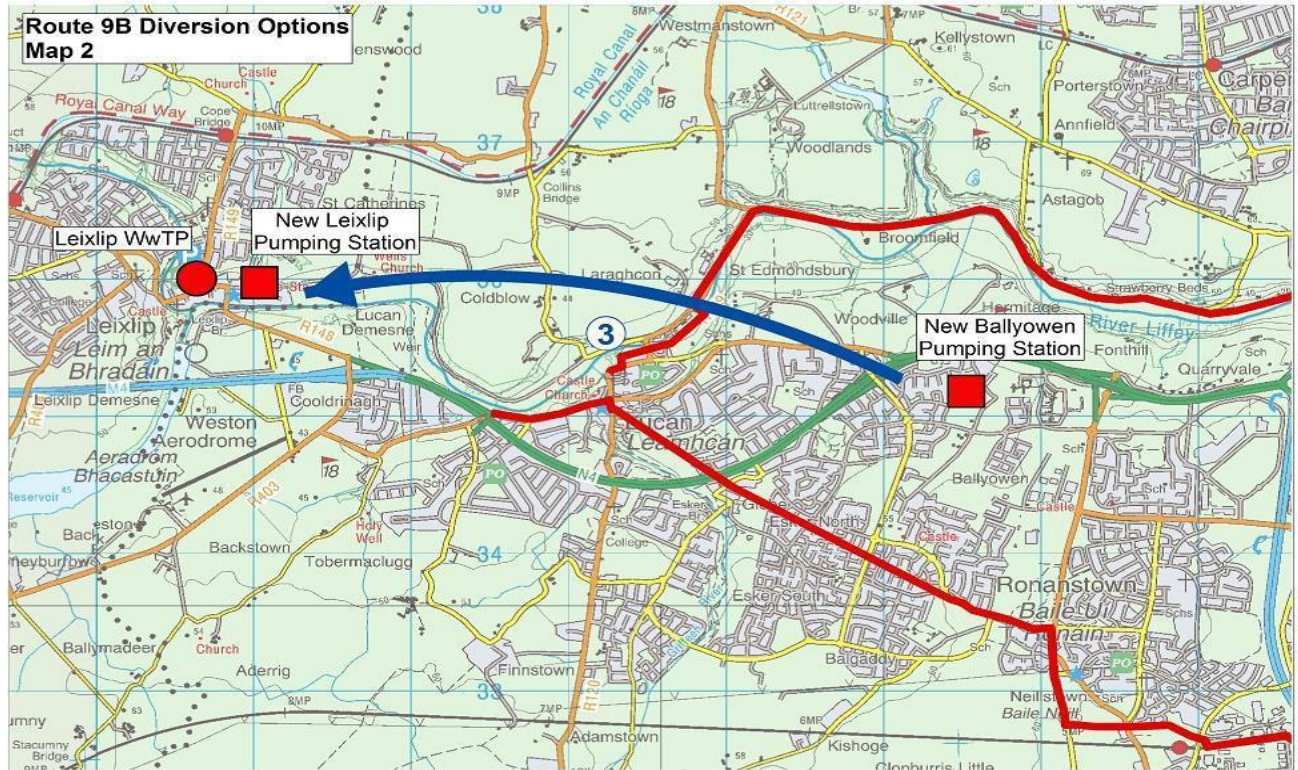
Figure 8.7 Duplication Options for 9B Sewer to GCTS



Should it be necessary to ultimately divert flows to the Orbital Sewer then a pumped option for the flows from the Lucan/ Clondalkin area of the 9B Catchment as shown on Figure 8.8 should be considered. It should be noted that significant constraints currently exist for this option including:

- Crossing of the River Liffey;
- Crossing of the Royal Canal and Dublin-Sligo Rail Line;
- Potential pipe routes constrained by existing pipelines and encroaching urbanisation; and
- Capacity of 9C sewer to accommodate additional flows

Figure 8.8 Connection Options for Lucan/ Clondalkin Catchment to Orbital Sewer



It is recommended that these options be explored in more detail through a DAP study of the 9B Sewer Catchment.

9. Conclusions and Recommendations

9.1 Conclusions

Domestic and Non-Domestic load on Ringsend WwTP will continue to grow under the growth scenarios examined.

The projected load development on Ringsend WwTP under the ‘most likely’ growth scenario indicates that the treatment capacity of 2.4 million PE to be provided at Ringsend WwTP will be exceeded in 2024. At 2050 this exceedance is projected at 508,024 PE.

Therefore, it will be necessary to divert flow and load out of the Ringsend catchment to the proposed Regional WwTP in order to maintain the loading on Ringsend WwTP below its treatment capacity of 2.4 million PE.

Diversion of the Blanchardstown (9C Sewer) catchment, inclusive of flows transferred to it from Leixlip WwTP, and the NFS sub-catchments located west of the proposed Regional WwTP will provide the necessary diversions from the Ringsend catchment to maintain the loading on Ringsend WwTP below 2.4 million PE.

It is feasible to divert wastewater from these catchments to the new Regional WwTP.

9.2 Recommendations

In developing the load transfer to the proposed Regional WwTP for planning purposes it is recommended that the ‘most likely’ growth scenario be used.

It is therefore recommended that the GDD project be designed to provide a wastewater treatment capacity of 500,000 PE.

Prudent planning suggests that load diversion from Ringsend WwTP commences before its treatment capacity is exceeded. Therefore, it is recommended that flow diversions commence as set out hereunder:

- 9C Sewer Catchment upstream of the M50 at 2025 (including the transferred flows from Leixlip); and
- NFS sub-catchments located west of the proposed Regional WwTP.

The required load diversions from the Ringsend Catchment would be satisfied at all stages up to and beyond 2050 (the design year horizon) by diverting the wastewater load generated in each of the above sub-catchments.

9.3 Projected Utilisation of Treatment Capacity provided at Regional WwTP

The projected utilisation of the treatment capacity to be provided at the Regional WwTP proposed under the GDD project is set out in Table 9.1 out to year 2050.

Table 9.1 Development of Required Treatment Capacity at Proposed Regional WwTP

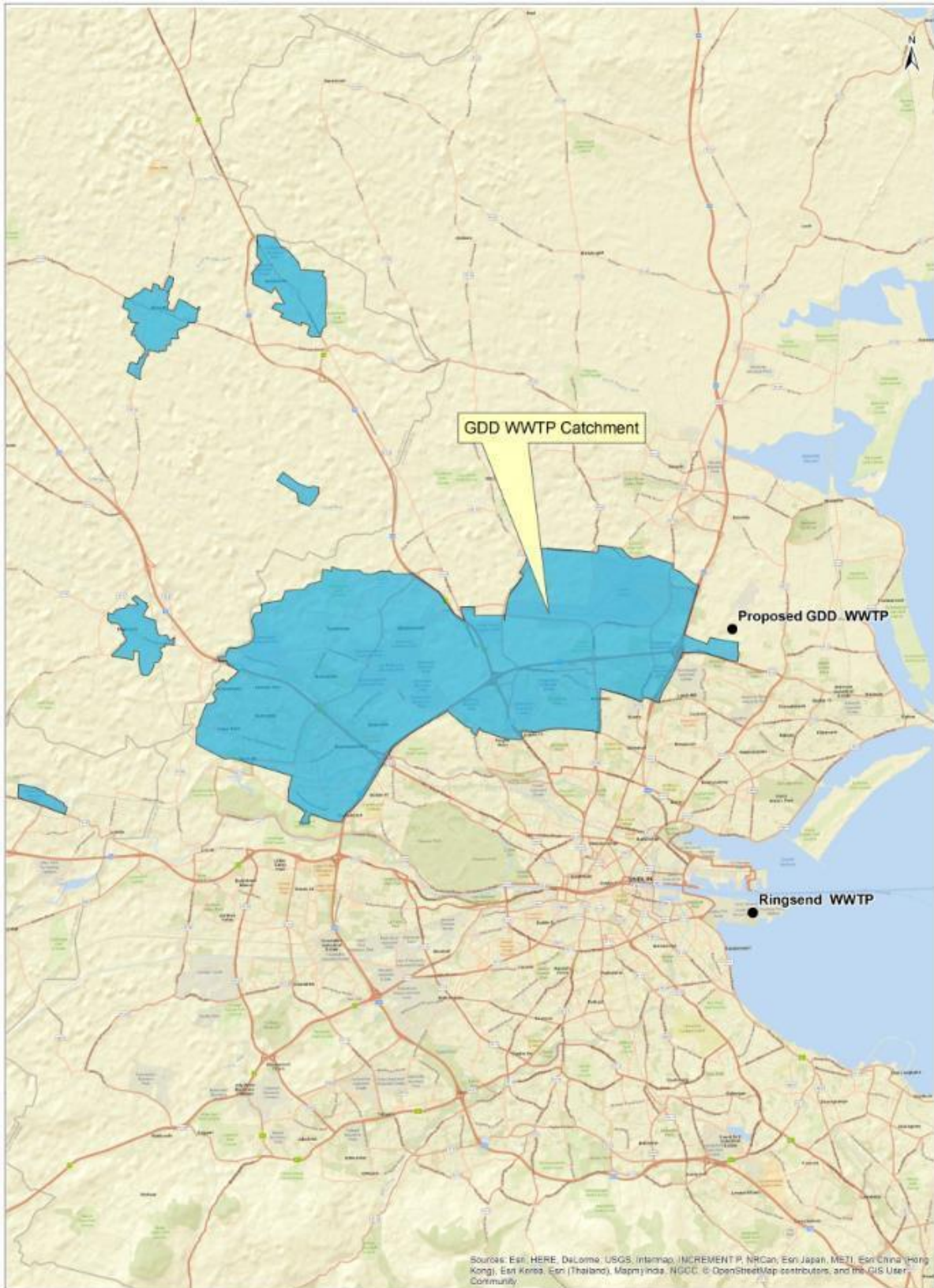
	Design Year			
	2025	2031	2040	2050
9C Sewer, incl load transferred from Leixlip WwTP	363,385	376,151	392,697	411,939
NFS sub-catchments west of proposed Regional WwTP	75,036	79,089	84,703	89,973
Total Treatment Capacity Required	438,421	455,240	477,400	501,912

The proposed 500,000 PE treatment capacity at the Regional WwTP will provide the projected treatment capacity requirements out to 2050 (the design year horizon for the GDD project).

9.4 Catchment of the Regional Wastewater Treatment Plant

The catchment of the proposed Regional WwTP is illustrated in Figure 9.1 below.

Figure 9.1 Catchment of the Proposed Regional WwTP



Appendix A. Loading Analysis on Wastewater Treatment Plants

9B (Lucan/Clondalkin) Sewer

9B Sewer		Growth Scenario 1 - Low		2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035	2036	2037	2038	2039	2040	2041	2042	2043	2044	2045	2046	2047	2048	2049	2050		
Lucan/Clondalkin	0.75% (2016 - 2021); 0.79% (2021 - 2031); 0.7% (2031 - 2041); 0.66% (2041 - 2050);	Population		72,289	72,831	73,377	73,928	74,482	75,041	75,634	76,231	76,833	77,440	78,052	78,669	79,290	79,917	80,548	81,184	81,753	82,325	82,901	83,481	84,066	84,654	85,247	85,844	86,444	87,050	87,624	88,202	88,785	89,371	89,960	90,554	91,152	91,753	92,35		
	16% of Res. Pop.	Commercial		11,566	11,653	11,740	11,828	11,917	12,007	12,101	12,197	12,293	12,390	12,488	12,587	12,686	12,787	12,888	12,989	13,080	13,172	13,264	13,357	13,451	13,545	13,639	13,735	13,831	13,928	14,020	14,112	14,206	14,299	14,394	14,489	14,584	14,681	14,77		
	No Growth	Industrial		1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400		
	Headroom (20% of P+C)						17,151	17,280	17,409	17,547	17,686	17,825	17,966	18,108	18,251	18,395	18,541	18,687	18,835	18,967	19,099	19,233	19,368	19,503	19,640	19,777	19,916	20,055	20,196	20,329	20,463	20,598	20,734	20,871	21,009	21,147	21,287	21,42		
Total 9B (Lucan/Clondalkin) Sewer				85,255	85,884	86,518	87,152	87,799	88,457	89,117	89,779	90,444	91,112	91,783	92,458	93,137	93,819	94,505	95,195	95,889	96,587	97,289	97,995	98,705	99,419	100,137	100,859	101,585	102,315	103,049	103,787	104,529	105,275	106,025	106,778	107,535	108,296	109,061	109,829	110,600

9B Sewer		Growth Scenario 2 - High		2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035	2036	2037	2038	2039	2040	2041	2042	2043	2044	2045	2046	2047	2048	2049	2050		
Lucan/Clondalkin	0.85% (2016 - 2021); 1.02% (2021 - 2031); 0.84% (2031 - 2041); 0.56% (2041 - 2050);	Population		72,289	72,903	73,523	74,148	74,778	75,414	76,053	76,696	77,345	78,000	78,658	79,320	80,000	80,685	81,385	82,090	82,800	83,515	84,235	84,960	85,690	86,425	87,165	87,910	88,660	89,415	90,175	90,940	91,710	92,485	93,265	94,050	94,840	95,635	96,435		
	16% of Res. Pop.	Commercial		11,566	11,665	11,764	11,864	11,965	12,066	12,169	12,274	12,381	12,489	12,598	12,708	12,819	12,931	13,044	13,158	13,273	13,389	13,506	13,624	13,743	13,863	13,984	14,106	14,229	14,353	14,478	14,604	14,731	14,859	14,988	15,118	15,249	15,381	15,514		
	No Growth	Industrial		1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400		
	Headroom (20% of P+C)						17,202	17,349	17,496	17,674	17,855	18,037	18,221	18,407	18,594	18,784	18,976	19,169	19,365	19,527	19,692	19,857	20,024	20,192	20,362	20,533	20,705	20,879	21,054	21,172	21,291	21,410	21,530	21,651	21,772	21,894	22,016	22,14		
Total 9B (Lucan/Clondalkin) Sewer				85,255	85,968	86,687	87,411	88,140	88,874	89,613	90,357	91,106	91,860	92,619	93,388	94,168	94,958	95,758	96,568	97,388	98,218	99,058	99,908	100,768	101,638	102,518	103,408	104,308	105,218	106,138	107,068	108,008	108,958	109,918	110,888	111,868	112,858	113,858	114,868	115,888

9B Sewer		Growth Scenario 3 - Most Likely		2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035	2036	2037	2038	2039	2040	2041	2042	2043	2044	2045	2046	2047	2048	2049	2050		
Lucan/Clondalkin	0.96% (2016 - 2021); 1.09% (2021 - 2031); 0.84% (2031 - 2041); 0.81% (2041 - 2050);	Population		72,289	72,983	73,684	74,391	75,105	75,826	76,553	77,288	78,033	78,788	79,553	80,328	81,113	81,908	82,713	83,528	84,353	85,188	86,033	86,888	87,753	88,628	89,513	90,408	91,313	92,228	93,153	94,088	95,033	96,008	96,993	97,988	98,993	99,993			
	16% of Res. Pop.	Commercial		11,566	11,677	11,789	11,903	12,017	12,132	12,264	12,398	12,533	12,670	12,808	12,948	13,089	13,231	13,376	13,521	13,635	13,749	13,865	13,981	14,099	14,217	14,337	14,457	14,579	14,701	14,820	14,940	15,061	15,183	15,306	15,430	15,555	15,681	15,80		
	No Growth	Industrial		1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400	1,400		
	Headroom (20% of P+C)						17,250	17,424	17,592	17,783	17,977	18,173	18,371	18,572	18,774	18,979	19,185	19,395	19,606	19,771	19,937	20,104	20,273	20,443	20,615	20,788	20,963	21,139	21,317	21,489	21,663	21,839	22,016	22,194	22,374	22,555	22,738	22,92		
Total 9B (Lucan/Clondalkin) Sewer				85,255	86,060	86,873	87,695	88,526	89,366	90,215	91,073	91,940	92,816	93,701	94,595	95,498	96,409	97,328	98,255	99,190	100,133	101,084	102,043	103,010	103,985	104,968	105,959	106,958	107,965	108,980	110,003	111,034	112,073	113,120	114,175	115,238	116,309	117,388	118,475	119,569

North Fringe Sewer - Phase 1 Diversion

North Fringe Sewer (Sub-catchments west of WwTP)		Growth Scenario 3 - Most Likely		2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035	2036	2037	2038	2039	2040	2041	2042	2043	2044	2045	2046	2047	2048	2049	2050	2051	2052	2053	2054	2055	2056	2057	2058	2059	2060	2061	2062	2063	2064	2065	
Fingal	0.96% (2016 - 2021); 1.09% (2021 - 2031); 0.84% (2031 - 2041); 0.81% (2041 - 2050);	Population	39,548	39,928	40,311	40,698	41,089	41,483	41,835	42,382	42,854	43,322	43,794	44,271	44,754	45,241	45,735	46,233	46,621	47,013	47,408	47,806	48,208	48,613	49,021	49,433	49,848	50,267	50,674	51,084	51,498	51,915	52,336	52,760	53,187	53,618	54,052	54,480	54,911	55,376	55,825	56,277	56,733	57,193	57,656	58,123	58,594	59,068	59,547	60,029	60,515	61,005		
	16% of Res. Pop.	Commercial	6,328	6,388	6,450	6,512	6,574	6,637	6,710	6,783	6,867	6,931	7,007	7,083	7,161	7,239	7,318	7,397	7,459	7,522	7,585	7,649	7,713	7,778	7,843	7,909	7,976	8,043	8,108	8,174	8,240	8,306	8,374	8,442	8,510	8,579	8,648	8,718	8,789	8,860	8,932	9,004	9,077	9,151	9,225	9,300	9,375	9,451	9,527	9,605	9,682	9,761		
	No Growth	Industrial	7,232	7,232	7,232	7,232	7,232	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732	14,732			
	Headroom (20% of P+C)					9,442	9,533	9,624	9,729	9,835	9,942	10,051	10,160	10,271	10,383	10,496	10,610	10,726	10,816	10,907	10,999	11,091	11,184	11,278	11,373	11,468	11,565	11,662	11,756	11,852	11,948	12,044	12,142	12,240	12,339	12,439	12,540	12,642	12,744	12,847	12,951	13,056	13,162	13,269	13,376	13,484	13,594	13,704	13,815	13,927	14,040	14,153		
Total North Fringe Sewer				53,108	53,548	53,993	54,444	54,899	55,354	55,809	56,264	56,719	57,174	57,629	58,084	58,539	58,994	59,449	59,904	60,359	60,814	61,269	61,724	62,179	62,634	63,089	63,544	63,999	64,454	64,909	65,364	65,819	66,274	66,729	67,184	67,639	68,094	68,549	69,004	69,459	69,914	70,369	70,824	71,279	71,734	72,189	72,644	73,099	73,554	74,009	74,464	74,919	75,374	75,829

Malahide WwTP

Malahide WwTP	Growth Scenario 1 - Low	2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035	2036	2037	2038	2039	2040	2041	2042	2043	2044	2045	2046	2047	2048	2049	2050	
0.75% (2016 - 2021); 0.79% (2021 - 2031); 0.7% (2031 - 2041); 0.66% (2041 - 2050); 16% of Residential Population No Industrial Load in Malahide No headroom allowed due to lack of industrial load.	PE Load	16,692	16,817	16,943	17,070	17,198	17,327	17,464	17,602	17,741	17,881	18,023	18,165	18,309	18,453	18,599	18,746	18,877	19,009	19,142	19,276	19,411	19,547	19,684	19,822	19,961	20,100	20,233	20,367	20,501	20,636	20,772	20,910	21,048	21,186	21,326	
	Commercial	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Industrial	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Total Malahide WwTP	16,692	16,817	16,943	17,070	17,198	17,327	17,464	17,602	17,741	17,881	18,023	18,165	18,309	18,453	18,599	18,746	18,877	19,009	19,142	19,276	19,411	19,547	19,684	19,822	19,961	20,100	20,233	20,367	20,501	20,636	20,772	20,910	21,048	21,186	21,326	

Malahide WwTP	Growth Scenario 2 - High	2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035	2036	2037	2038	2039	2040	2041	2042	2043	2044	2045	2046	2047	2048	2049	2050	
0.85% (2016 - 2021); 1.02% (2021 - 2031); 0.84% (2031 - 2041); 0.56% (2041 - 2050); 16% of Residential Population No Industrial Load in Malahide No headroom allowed due to lack of industrial load.	PE Load	16,692	16,834	16,977	17,121	17,267	17,414	17,591	17,771	17,952	18,135	18,320	18,507	18,696	18,886	19,079	19,274	19,435	19,599	19,763	19,929	20,097	20,266	20,436	20,607	20,781	20,955	21,072	21,190	21,309	21,428	21,548	21,669	21,790	21,913	22,035	
	Commercial	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Industrial	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Total Malahide WwTP	16,692	16,834	16,977	17,121	17,267	17,414	17,591	17,771	17,952	18,135	18,320	18,507	18,696	18,886	19,079	19,274	19,435	19,599	19,763	19,929	20,097	20,266	20,436	20,607	20,781	20,955	21,072	21,190	21,309	21,428	21,548	21,669	21,790	21,913	22,035	

Malahide WwTP	Growth Scenario 3 - Most Likely	2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035	2036	2037	2038	2039	2040	2041	2042	2043	2044	2045	2046	2047	2048	2049	2050	
0.96% (2016 - 2021); 1.09% (2021 - 2031); 0.84% (2031 - 2041); 0.81% (2041 - 2050); 16% of Residential Population No Industrial Load in Malahide No headroom allowed due to lack of industrial load.	PE Load	16,692	16,852	17,014	17,177	17,342	17,509	17,700	17,893	18,088	18,285	18,484	18,685	18,889	19,095	19,303	19,514	19,677	19,843	20,009	20,178	20,347	20,518	20,690	20,864	21,039	21,216	21,388	21,561	21,736	21,912	22,089	22,268	22,449	22,631	22,814	
	Commercial	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Industrial	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Total Malahide WwTP	16,692	16,852	17,014	17,177	17,342	17,509	17,700	17,893	18,088	18,285	18,484	18,685	18,889	19,095	19,303	19,514	19,677	19,843	20,009	20,178	20,347	20,518	20,690	20,864	21,039	21,216	21,388	21,561	21,736	21,912	22,089	22,268	22,449	22,631	22,814	

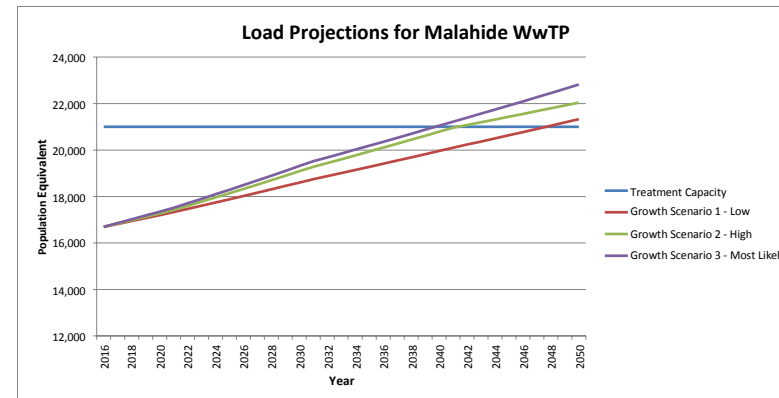
Malahide WwTP

Treatment Capacity

Growth Scenario 1 - Low
Growth Scenario 2 - High
Growth Scenario 3 - Most Likely

2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035	2036	2037	2038	2039	2040	2041	2042	2043	2044	2045	2046	2047	2048	2049	2050		
21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000
16,692	16,817	16,943	17,070	17,198	17,327	17,464	17,602	17,741	17,881	18,023	18,165	18,309	18,453	18,599	18,746	18,877	19,009	19,142	19,276	19,411	19,547	19,684	19,822	19,961	20,100	20,233	20,367	20,501	20,636	20,772	20,910	21,048	21,186	21,326		
16,692	16,834	16,977	17,121	17,267	17,414	17,591	17,771	17,952	18,135	18,320	18,507	18,696	18,886	19,079	19,274	19,435	19,599	19,763	19,929	20,097	20,266	20,436	20,607	20,781	20,955	21,072	21,190	21,309	21,428	21,548	21,669	21,790	21,913	22,035		
16,692	16,852	17,014	17,177	17,342	17,509	17,700	17,893	18,088	18,285	18,484	18,685	18,889	19,095	19,303	19,514	19,677	19,843	20,009	20,178	20,347	20,518	20,690	20,864	21,039	21,216	21,388	21,561	21,736	21,912	22,089	22,268	22,449	22,631	22,814		

Note: Average Daily Loading to Malahide WwTP for 2015 (Jan - Nov) is reported by IW at 16,344 PE



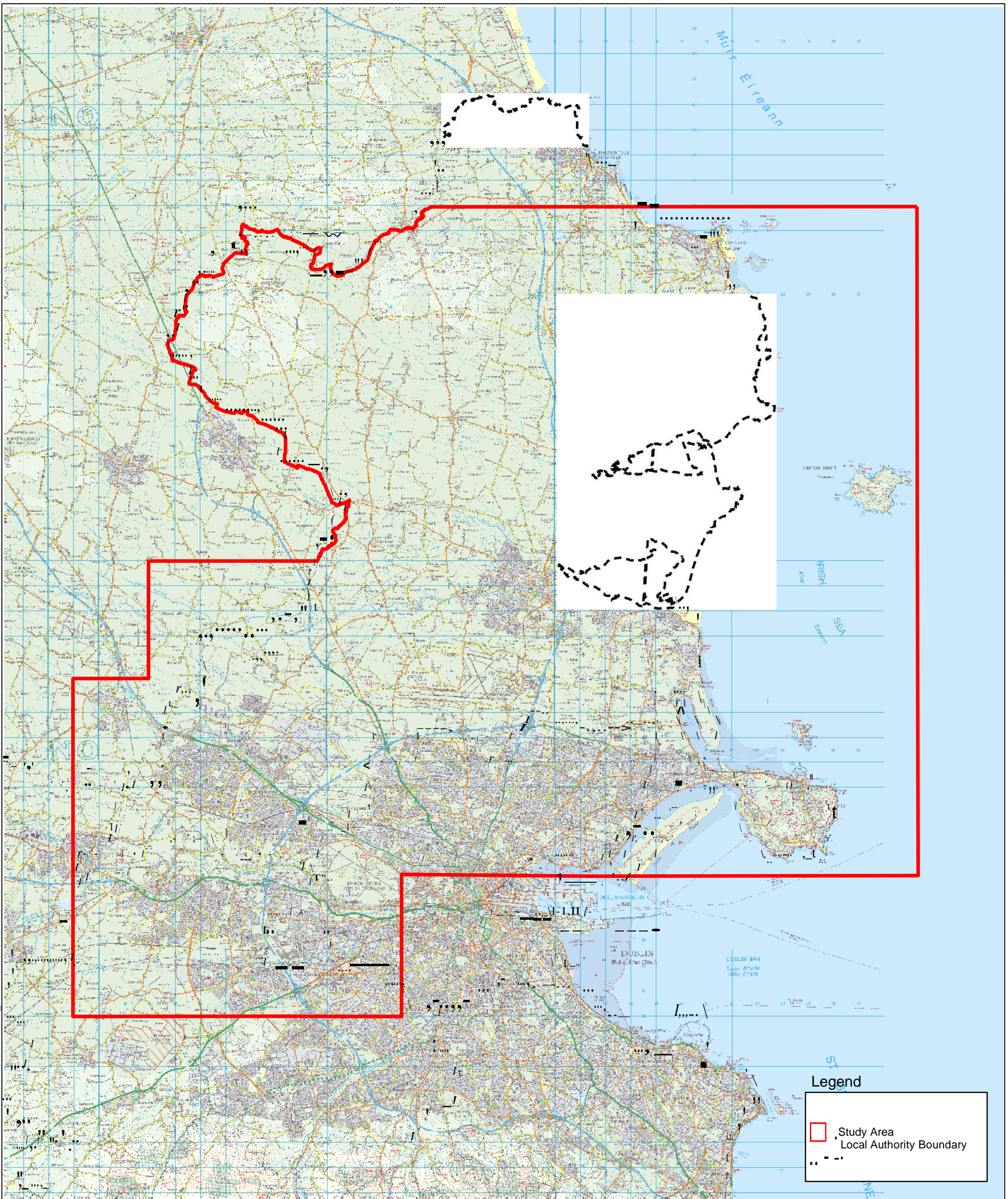
Malahide WwTP - Load Transfers to Regional WwTP

Treatment Capacity Malahide WwTP

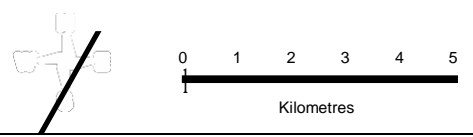
Growth Scenario 1 - Low
Growth Scenario 1 - High
Growth Scenario 2 - Most Likely

2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035	2036	2037	2038	2039	2040	2041	2042	2043	2044	2045	2046	2047	2048	2049	2050					
21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000	21,000			
								-3,259	-3,119	-2,977	-2,835	-2,691	-2,547	-2,401	-2,254	-2,123	-1,991	-1,858	-1,724	-1,589	-1,453	-1,316	-1,178	-1,039	-900	-767	-633	-499	-364	-228	-90	48	186	326					
								-3,229	-3,048	-2,865	-2,680	-2,493	-2,304	-2,114	-1,921	-1,726	-1,565	-1,401	-1,237	-1,071	-903	-734	-564	-393	-219	-45	72	190	309	428	548	669	790	913	1,035				
								-3,300	-3,107	-2,912	-2,715	-2,516	-2,315	-2,111	-1,905	-1,697	-1,486	-1,323	-1,157	-991	-822	-653	-482	-310	-136	39	216	388	561	736	912	1,089	1,268	1,449	1,631	1,814			
							Sc.1	0%	-16%	-15%	-14%	-13%	-13%	-12%	-11%	-11%	-10%	-9%	-9%	-8%	-8%	-7%	(10%-20% overcapacity is acceptable)																
							Sc.2	-15%	-15%	-14%	-13%	-12%	-11%	-10%	-9%	-9%	-8%	-8%	-7%	(10%-20% overcapacity is acceptable)																			
								-15%	-14%	-13%	-12%	-11%	-10%	-9%	-8%	-8%	-7%	(10%-20% overcapacity is acceptable)																					

Appendix B. Drawings



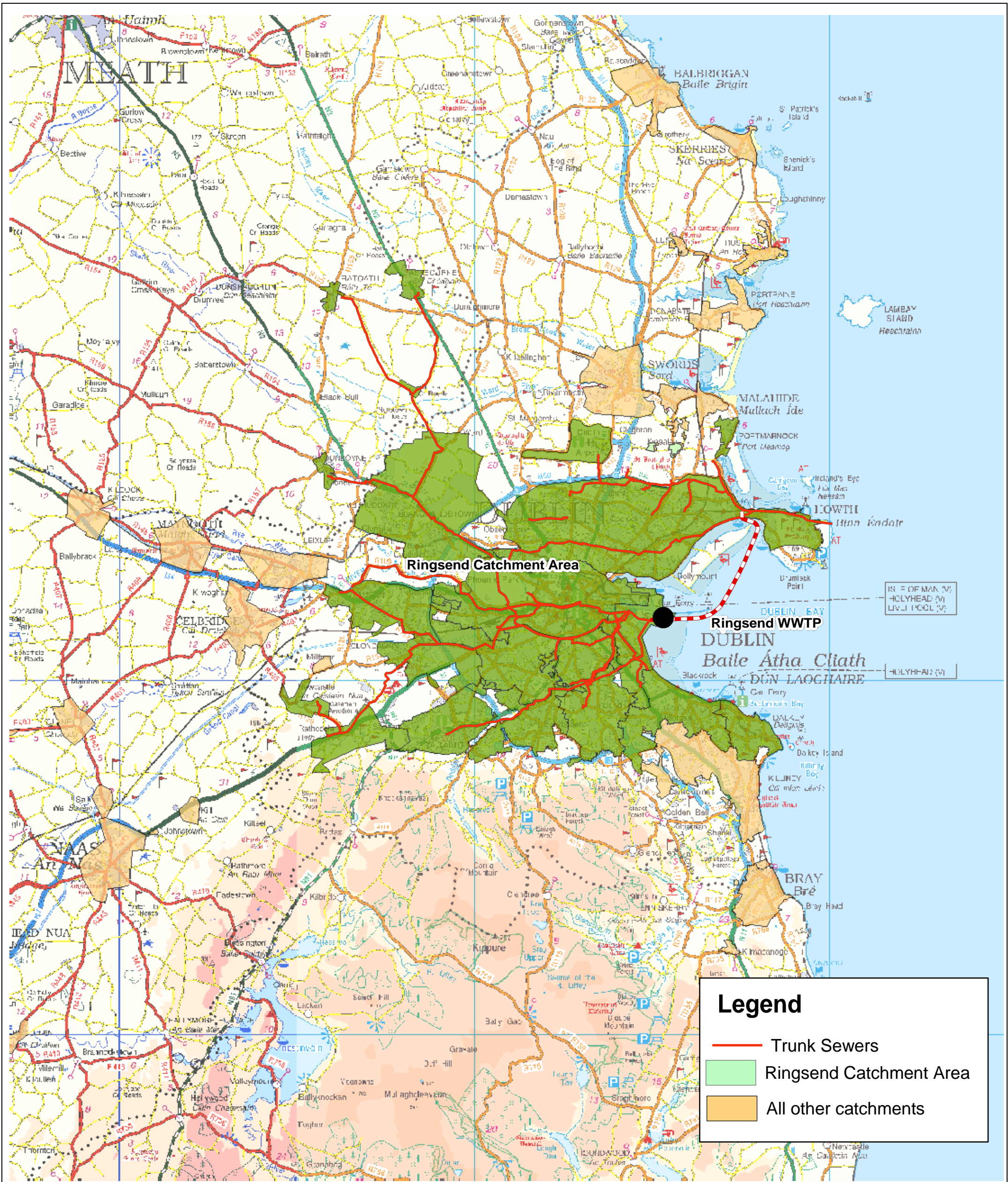
Rev.	Date	Purpose of revision	Drawn	Checked	Rev'd	App'd
C	10/05/12	ISSUED	GF	OM	JB	COK
B	01/12/11	ISSUED	AG	ED	JB	COK
A	19/09/11	ISSUED	AG	ED	JB	COK



Drawing Title
GREATER DUBLIN DRAINAGE STUDY AREA

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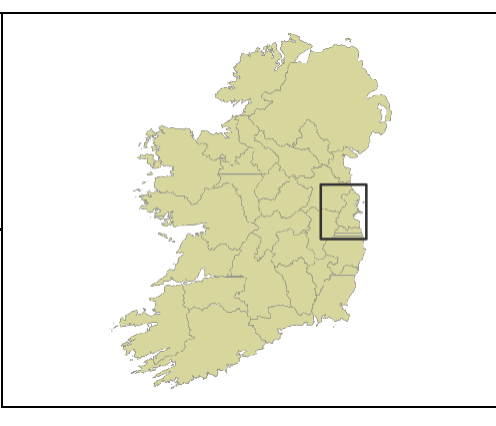
Drawing Status: **ISSUED**
 Scale @ A3: 1:140,000
 Job No.: 32102900
 Drawing No.: **FIGURE 2.1**



Greater Dublin Drainage

Drawing Title

RINGSEND CATCHMENT AREA



Rev.	Date	Purpose of revision	Drawn	Checked	Rev'd	App'd
A	12/07/11	ISSUED	JenC	NC	JB	KO

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Scale: 0 0.95 1.9 2.85 3.8 4.75 Kilometres

JACOBS **TOBIN**

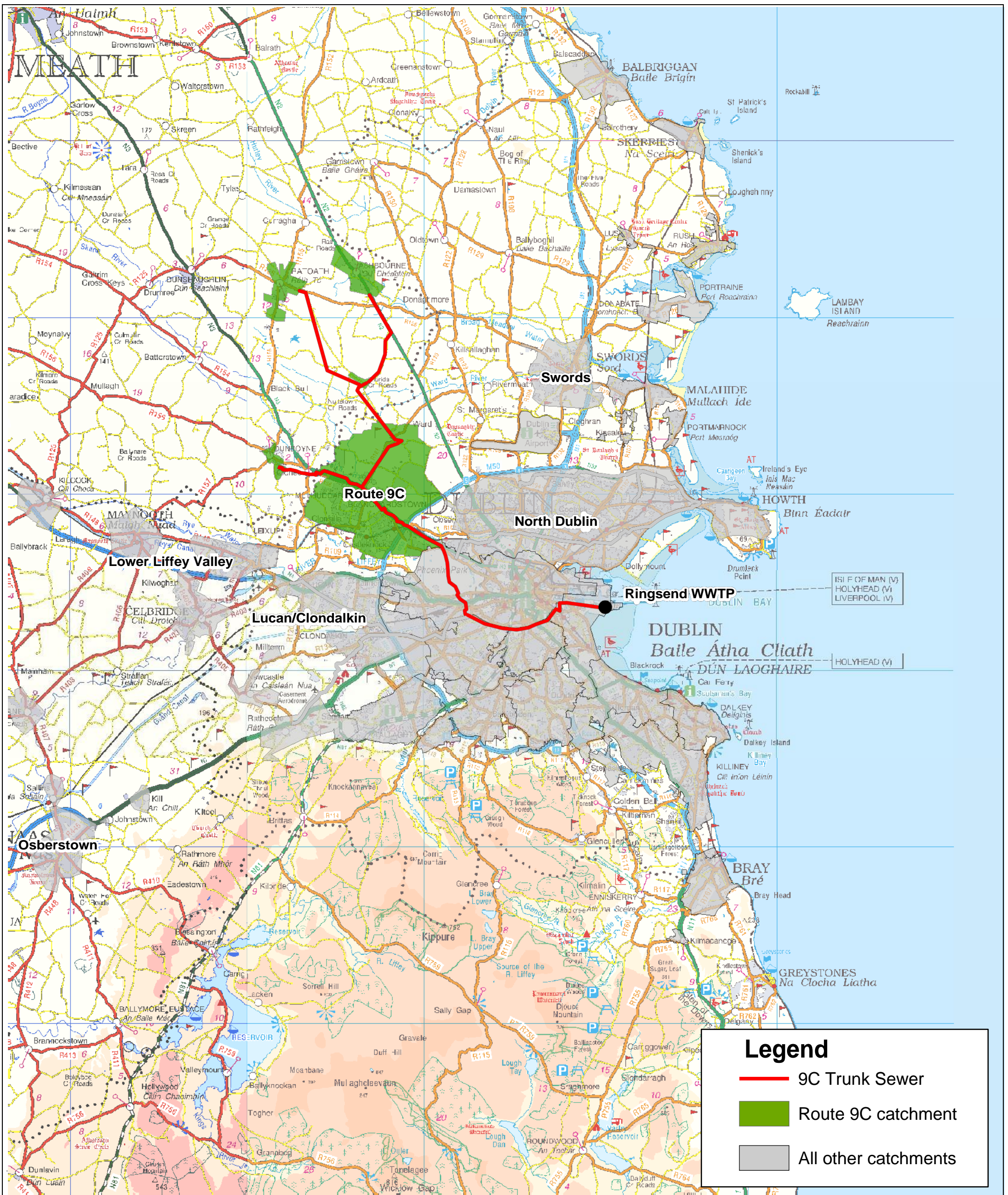
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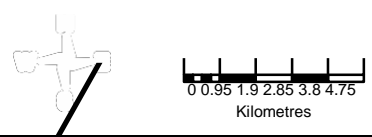
Job No.: 32102900

Drawing No.: **FIGURE 2.2**

DO NOT SCALE



Rev.	Date	Purpose of revision	Drawn	Check'd	Rev'd	App'd
A	12/07/11	ISSUED	JenC	NC	JB	KO



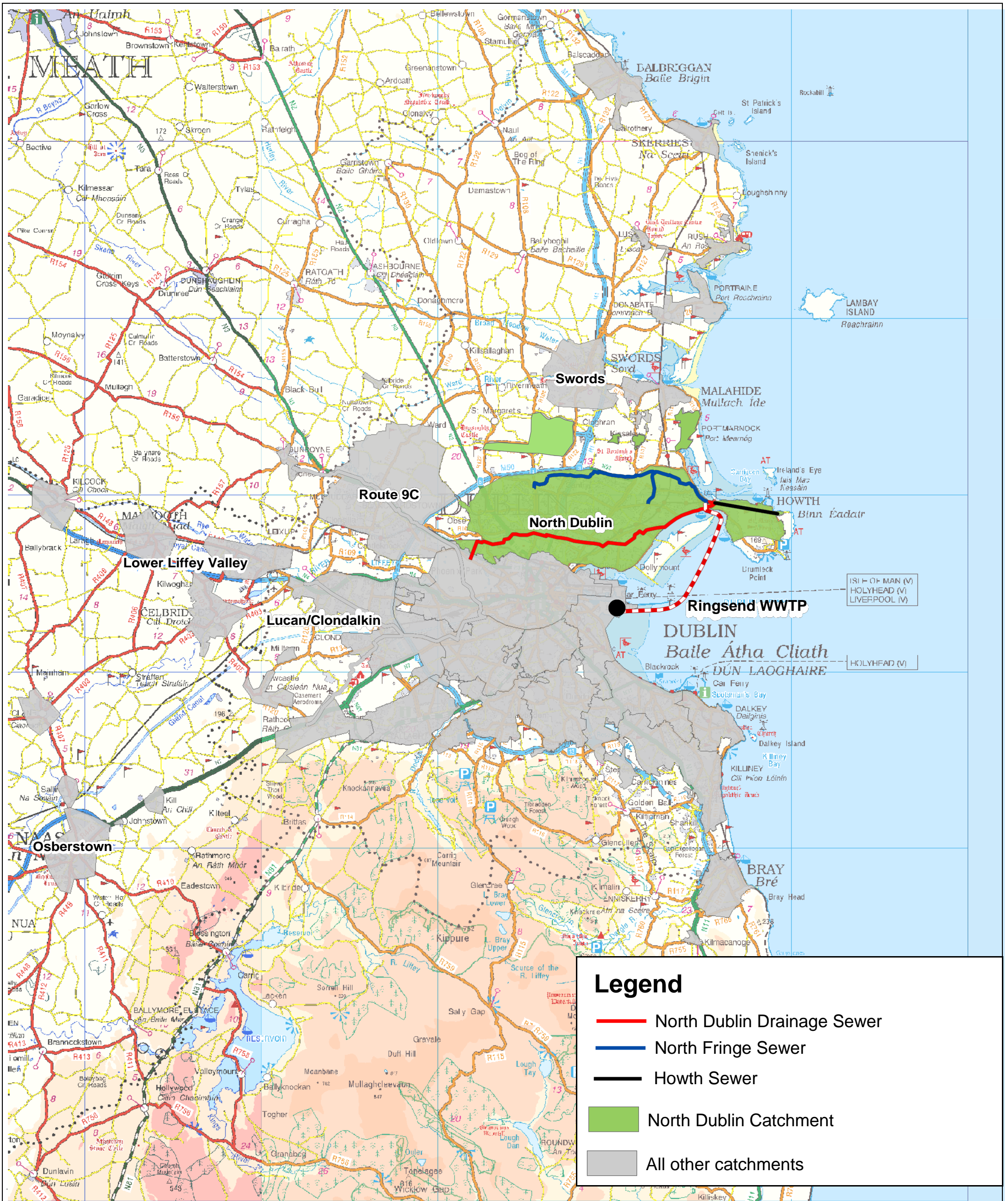
ROUTE 9C CATCHMENT

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Drawing Status	ISSUED	
Scale @ A3	1:200,000	DO NOT SCALE
Job No.	32102900	

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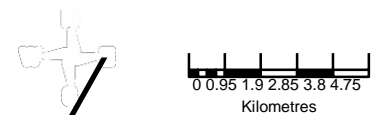
FIGURE 2.3



Rev.	Date	Purpose of revision	Drawn	Check'd	Rev'd	App'd
A	12/07/11	ISSUED	JenC	NC	JB	KO

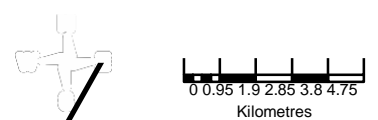
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Drawing Status	ISSUED	
Scale @ A3	1:200,000	DO NOT SCALE
Job No.	32102900	

Drawing No. **FIGURE 2.4**



Drawing Title

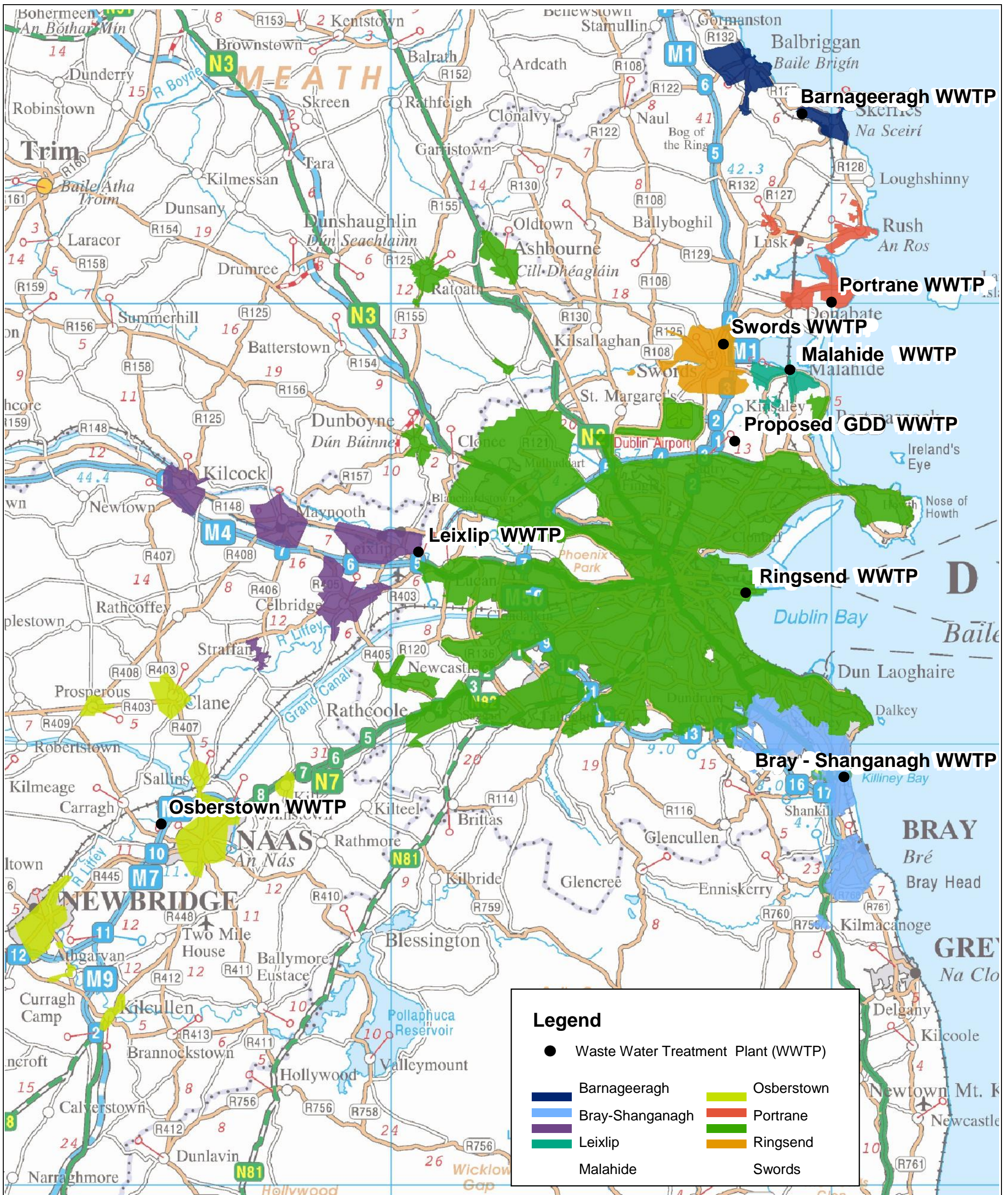
ROUTE 9B CATCHMENT

Rev.	Date	Purpose of revision	Drawn	Check'd	Rev'd	App'd
A	12/07/11	ISSUED	JenC	NC	JB	KO

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Drawing Status	ISSUED	
Scale @ A3	1:200,000	DO NOT SCALE
Job No.	32102900	
Drawing No.	FIGURE 2.5	



Greater Dublin Drainage

Map Title: **KEY WASTEWATER TREATMENT STANDARDS**

WASTEWATER TREATMENT SITES & CATCHMENTS



Rev.	Date	Purpose of revision	Drawn	Checked	Revised	Approved
A	16/05/2018	Final	RG	SmG	PW	CoK

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Scale: 0 1 2 3 4 5 6 Kilometres

JACOBS **TOBIN**

File Path: Y:\GDD\Mxds\LoadAssessment\Figure2.6_WWTP_SitesandCatchments.mxd

Drawing Status: **ISSUED**

Scale @ A3: 1:200,000

Job No.: 32102902

Drawing No.: **Figure 2.6**

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